

MINNA NATOMA ART CORRIDOR

PUBLIC ART PROJECT

Street Furniture: Masako Miki and Jesse Schlesinger

Street Surface Paving: Barbara Stauffacher Solomon, Mel Vera Cruz, and Muzae Sesay

Visual Arts Committee
July 15, 2026

Arianne G. Davidian
Project Manager | Public Art
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sfac san francisco
arts commission

MINNA NATOMA ART CORRIDOR

CONSTRUCTION DOCUMENTATION

Street Furniture: Masako Miki and Jesse Schlesinger (attached)

STRUCTURAL ABBREVIATIONS			
B.O.T.	BOTTOM OF	R.G.D.	REQUIRED
B.O.T.	BOTTOM	SEE ARCHITECTURAL DWGS	
CONC.	CONCRETE	S.L.D.	SEE LANDSCAPE DWGS
CONN.	CONNECTION	S.M.S.	SHEET METAL SCREW
COUNTINGCTGCTK	COUNTING CONT.	STD.	STANDARD
D.WGS.	DRAWINGS	T.O.C.	TOP OF CONCRETE
(E)	EXISTING	T.O.S.	TOP OF STEEL
EA.	EACH	TPP.	TYPICAL
E.A.	EACH SIDE	U.O.N.	UNLESS OTHERWISE NOTED
E.O.R.	ENGINEER OF RECORD	V.I.F.	VERIFY IN FIELD
E.S.	EACH WAY	W/	WITH
F.F.S.	FINISH FLOOR FINISH	X/S	EXTRA STRONG (PIPE)
H.D.G.	HOT-DIP GALVANIZED	X/S	DOUBLE EXTRA STRONG (PIPE)
H.S.	HEADED STUDS	Δ	AND
HSS	HOLLOW STRUCTURAL SECTION	CL	CENTER LINE
GA	GALVNEE	Ø	DIAMETER OR ROUND
OW	ANGLE (STEEL SHAPE)	PLATE.	PROPERTY LINE
LG	LIGHT WEIGHT	N	NUMBER OR NUMBER
MAX.	MAXIMUM (AT MOST)	±	TOLERANCE
MIN.	MINIMUM (AT LEAST)		
(N)	NEW		
O.C.	ON CENTER		
P.D.F.	POWDER DRIVEN FASTENER(S)		

FOR POWDER-DRIVEN FASTENERS AND CONCRETE ANCHOR ABBREVIATIONS, SEE HILTI NORTH AMERICAN PRODUCT TECHNICAL GUIDE (available at www.us.hilti.com) AND SIMPSON STRONG-TIE ANCHOR SYSTEMS CATALOG (available at www.strongtie.com).

INSTALLATION NOTES:

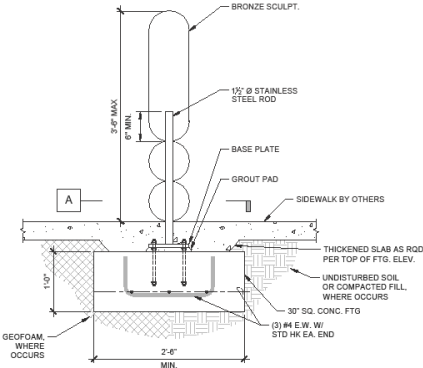
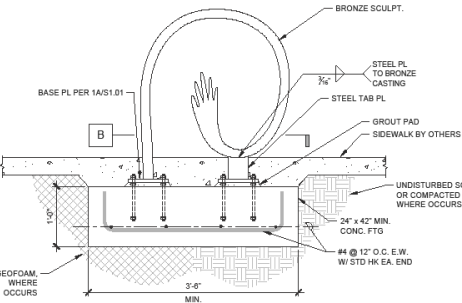
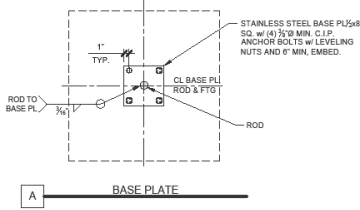
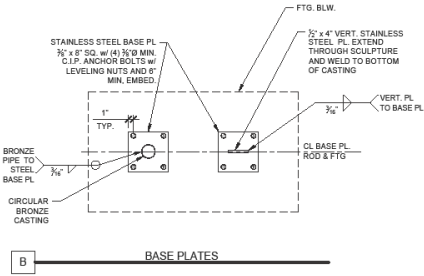
- 1) ALL CAST-IN-PLACE CONCRETE SHALL BE PLACED ACCORDING TO THE MINIMUM THICKNESS INDICATED ON THE DRAWINGS.
- 2) CONCRETE REINFORCEMENT SHALL HAVE MINIMUM 3" CLEAR COVER IN LOCATIONS CAST AGAINST EARTH, 1.5" CLEAR COVER AT OTHER LOCATIONS

ADDITIONAL NOTES:

- 1) ALL CAST-IN-PLACE CONCRETE SHALL BE NORMAL WEIGHT WITH A MINIMUM SPECIFIED COMPRESSIVE STRENGTH f'_c OF 2500PSI.
- 2) ALL CAST-IN-PLACE STEEL REINFORCING BARS SHALL CONFORM TO ASTM A618, GR 60 OR EQUIVALENT.
- 3) THREADED ANCHOR RODS WITH NUTS SHALL BE STAINLESS STEEL AND MEET ASTM A193 GR B8 WITH A MIN VIELD STRENGTH OF 30KSI
- 4) BRONZE MATERIAL SHALL BE 2% ALUMINUM, SILICON BRONZE OR EQUAL, WITH MINIMUM YIELD STRENGTH, F_y = 24 KSI.
- 5) STAINLESS STEEL BASE PLATES SHALL CONFORM TO ASTM A240 GR 304 WITH A MINIMUM YIELD STRENGTH OF 30KSI
- 6) NONSKIRM CRACK, INSTALLED PER MANUFACTURER'S RECOMMENDATIONS SHALL COMPLY WITH ASTM STANDARD C1107 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,400 PSI IN 48 HOURS AND 5,000 PSI IN 28 DAYS

SPECIAL INSPECTION NOTES:

- 1) SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT 3RD PARTY INSPECTOR HIRED BY THE OWNER AS FOLLOWS:
 - A) EXCAVATION AND PLACEMENT OF REINFORCING STEEL AND CONCRETE.
 - B) MATERIAL TESTING OF CONCRETE IS NOT REQUIRED



2 BIKE RACK SCULPTURE
\$1.01 ELEVATION 1" = 1'-0"

1
S1.01

BOLLARD SCULPTURE
ELEVATION

1" = 1'-0"



DRAWINGS AND WRITTEN MATERIAL APPEARING HEREIN
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REVISIONS	DATE	BY
FOR CONSTRUCTION	4-29-2028	
FOR CONSTRUCTION	6-12-2028	

MASAKO MIKI SCULPTURES
MINNA NATOMA ART WALK
SAN FRANCISCO, CA

DATE:	3-19-26
SCALE:	AS NOTED
JOB NO.:	2024.308.00

SHEET TITLE:
STRUCTURAL

SHEET NUMBER:

S1.01

Minna Natoma Art Corridor
Construction Documentation
Masako Miki



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GENERAL NOTES

1. ALL WORK AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2023 CALIFORNIA BUILDING CODE (REFERRED TO HEREINAFTER AS "CBC") AND 2023 SAN FRANCISCO BUILDING CODE (REFERRED TO HEREINAFTER AS "SBC").
2. ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE, UNLESS NOTED OTHERWISE. NOTES AND DETAILS ON THE DRAWING THAT ARE NOT SHOWN SHALL BE CONSIDERED AS TYPICAL.
3. ALL OMISSIONS AND CONFLICTS BETWEEN VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR ARCHITECTURAL SPECIFICATIONS (WHERE APPLICABLE) SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY OF THE WORK INVOICED.
4. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE PROTECTION AND PRESERVATION OF EXISTING UTILITIES AND STRUCTURES AND FOR ALL NECESSARY NOTIFICATION ENGINEERING SERVICES OF THESE CONDITIONS. THE ARCHITECT'S OR ENGINEER'S REVIEW OF THE DRAWINGS IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.
5. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
6. IN NO CASE SHALL WORKING DIMENSIONS BE SCALED FROM PLANS, SECTIONS, DETAILS OR THE STRUCTURAL DRAWING.
7. NO OPENINGS, CHARGES, NOTCHES, ETC. SHALL BE PLACED IN COLUMNS, JOISTS, BEAMS, BRACING WALLS, AND SHEAR WALLS UNLESS SPECIFICALLY NOTED ON THESE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE ADEQUATE AGENCIES.
8. CONTRACTOR SHALL FOLLOW AND COMPLY WITH ALL MANUFACTURER'S GUIDELINES AND SPECIFICATIONS OF THE PRODUCTS INCLUDED IN THE DRAWINGS.
9. ALL ASTM DESIGNATIONS SHALL BE AS AMENDED TO DATE UNLESS NOTED OTHERWISE.
10. IT IS SOLELY THE CLIENT'S RESPONSIBILITY TO ENSURE THAT THE U.S. APPROVED MATERIALS LISTED IN THE GENERAL NOTES ARE USED AND THAT ANY SUBSTITUTES MEET THE APPROVED MATERIALS AND METHODS.

DESIGN CRITERIA

1. DEAD LOADS:
a. DEAD BENCH 100LBS
b. SMALL BENCH 400LBS EACH
2. LIVE LOADS: 200LBS
3. SEISMIC DESIGN PARAMETERS:
a. IMPORTANCE FACTOR $I = 1.0$
b. RISK CATEGORY II
c. SITE CLASS D
d. MAPPED SHORT PERIOD ACCELERATION $S_a = 1.5$
e. MAPPED SHORT PERIOD ACCELERATION $S_s = 1.98$
f. DESIGN SHORT PERIOD ACCELERATION $S_d = 1.120$
g. MAPPED SECOND ACCELERATION $S_a = 0.800$
h. DESIGN SECOND ACCELERATION $S_d = 1.153$
i. SEISMIC DESIGN CATEGORY D
- DESIGN BASE SHEAR - $V = C_w M$ AT STRENGTH LEVEL
($M =$ EFFECTIVE SEISMIC WEIGHT)
- j. SEISMIC RESPONSE COEFFICIENT $C_w = 0.560$
RESPONSE REDUCTION FACTOR $R = 2$
4. WIND DESIGN PARAMETERS:
a. BASIC WIND SPEED 90 MPH
b. RISK CATEGORY II
c. EXPOSURE CATEGORY C
d. WIND PRESSURES:
- MAIN WIND FORCE RESISTING SYSTEM: 16.0 PSF
5. FOUNDATION DESIGN PARAMETERS:
SPREAD FOOTING PARAMETERS:
a. ALLOWABLE SOIL PRESSURE:
- DEAD LOADS 2,500 PSF
- DEAD PLUS LIVE LOADS 2,200 PSF
- DEAD PLUS LIVE PLUS SEISMIC 2,000 PSF
b. COEFFICIENT OF FRICTION 0.30 (1.5% SAFETY FACTOR INCLUDED)

FOUNDATION

1. FOUNDATION DESIGN SHALL BE BASED ON THE PRESUMPTIVE LOAD BEARING VALUES OF SOIL GIVEN IN THE GSC, TABLE 1806.2.
2. INSTALLATION OF THE FOUNDATION FOOTINGS OR PILES WITH RESPECT TO THE DEPTH BELOW THE MEAN WATER TABLE SHALL BE TO A MINIMUM ACCORDING TO THE FOUNDATION DETAILS ON THESE PLANS. FIELD DISCOVERIES CONDITIONS MAY NECESSITATE DEEPER FOUNDATIONS.
3. EXCEPT WHERE OTHERWISE SHOWN, EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE MEAN LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE.
4. ALL EXCAVATIONS ARE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
5. ALL WATER, SOIL AND OTHER DEBRIS SHALL BE REMOVED FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING OF CONCRETE.
6. SUBGRADE AND BASE SHALL BE PREPARED PER THE GEOTECHNICAL REPORT; IF NO GEOTECHNICAL REPORT IS AVAILABLE, THE COMPACTED FILL DENSITY, 12 INCHES IN DEPTH OR MORE, MEASURED BY STANDARD TEST METHOD SHALL BE NOT LESS THAN 95 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED BY STANDARD TEST METHOD. THE MAXIMUM DRY DENSITY SHALL BE VERIFIED BY SPECIAL INSPECTION IN ACCORDANCE WITH THE SPECIAL INSPECTION SECTION.
7. ALL BACKFILL WITH ENGINEERED FILLS SHALL BE COMPACTED TO 95% RELATIVE DENSITY.

CONCRETE

1. ALL CONCRETE CONSTRUCTION SHALL BE PER CBC CHAPTER 18 AND IN ACCORDANCE WITH ACI 318-22, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
2. ALL CONCRETE SHALL HAVE A MINIMUM WATER-TO-CEMENT RATIO OF 0.48 FOR FOUNDATIONS AND ALL STRUCTURAL ELEMENTS AND FOR SLABS, "C-1" SLUMP, AND SHALL OBTAIN A 28 DAY MINIMUM COMPRESSIVE STRENGTH AS FOLLOWS:
 - a. FOOTINGS 2,500 PSI
3. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE, WEIGHING LESS THAN 150 PCF, UNLESS OTHERWISE NOTED. ALL CONCRETE FILL OVER METAL DECK SHALL BE LIGHTWEIGHT CONCRETE, WEIGHING LESS THAN 115 PCF, UNLESS OTHERWISE NOTED.
4. GENERAL SHALL CONFORM TO ASTM C150, TYPE I (OR ENGINEERED MAXIMUM DESIGN TO STRENGTH)
5. HARD ROCK AGGREGATES SHALL CONFORM TO ASTM C33. MAXIMUM NOMINAL SIZE OF AGGREGATE SHALL NOT EXCEED 1/4" FOR FOUNDATION CONCRETE AND 1" FOR ALL STRUCTURAL CONCRETE. ALL AGGREGATE SHALL CONFORM TO ALL AASHTO AND ACI STANDARD SPECIFICATIONS. MAXIMUM NOMINAL SIZE ALSO BE SELECTED SUCH THAT WORKABILITY AND PLACEMENT OF CONCRETE SHALL BE MAINTAINED.

10 GENERAL NOTES

- a. ALL ALTERNATE CONCRETE MIX DESIGN AND TEST STRENGTHS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.
7. MAXIMUM VERTICAL DROP OF CONCRETE SHALL BE NO MORE THAN 2'-0" FROM END OF PLACEMENT DEVICE TO PLACEMENT SURFACE.
8. CONCRETE COVER AT REINFORCING SHALL BE AS FOLLOWS:

a. CAST IN PLACE AND PERMANENTLY EXPOSED TO WEATHER	3" CLEAR
b. EXPOSED TO WEATHER BUT CAST AGAIN FORMING	2" CLEAR
c. SLABS (EXCEPT FOR MATS)	REBAR AT CENTER OF SLAB
d. SLABS PRECAST, TO COVER EXPOSED REBAR	1" CLEAR
e. NOT EXPOSED TO WEATHER OR EARTH SHAKES, WALLS, JOISTS	¾" CLEAR
f. NOT EXPOSED TO WEATHER OR EARTH SHAKES AND COLUMNS	1½" CLEAR
9. COMMERICAL GRADE SHRAPAGE-COMPENSATION OR NON-SHRINK PORTLAND CEMENT GROUT SHALL CONFORM TO ASTM C1121.
10. ALL REINFORCING STEEL, DOWNES, ANCHOR BOLTS, PIPE SLEEVES AND OTHER INSERTS SHALL BE INSTALLED IN PLACE OF CONCRETE. NOT SETTING SHALL NOT ALLOWED.
11. THE SURFACE OF ALL CONSTRUCTION JOINTS SHALL BE CLEANED AND ROUGHENED BY REMOVING THE ENTIRE SURFACE AND EXPOSING CLEAN AGGREGATE SLOTTED EMBEDDED IN MORTAR MIX.
12. EPOXY SET ANCHORS SHALL BE INSTALLED IN CONCRETE THAT HAS A MINIMUM AGE OF 21 DAYS PER ACI D05.2.
13. INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERSONNEL OR EMPLOYEE CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE AD/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. THE ACCEPTABILITY OF CERTIFICATION OTHER THAN AD/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION SHALL BE DETERMINED BY THE DESIGNING PROFESSIONAL.

REINFORCING BAR

1. REINFORCING STEEL SHALL BE DEFINED BARS PER ASTM A36 WITH BAR MARKS LEGIBLY ROLLED INTO THE SURFACE. INDICATION SIZE, TYPE OF STEEL, AND YIELD STRENGTH DESIGNATION:
 - A3 BARS AND LARGER GRADE 60 OR GRADE 65
 - ALL BARS TO BE WELDED GRADE 60
 - ALL BARS TO BE WELDED GRADE 60
2. REINFORCING SHALL HAVE A MINIMUM LAP IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS THROUGH THE THICK DRIVING. STAGGER SPACING SHALL BE AS POSSIBLE. VERTICAL LAP REINFORCING BARS SHALL EXTEND INTO FOOTINGS OF LAP SPACING WITH FOOTING DOWNWARDS. ALL BARS SHALL BE WELDED.
3. REINFORCING OF REINFORCING SHALL BE IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS SHOWN ON THESE DRAWINGS. FIELD BENDING OF BARS THAT ARE IN PLACE IS NOT PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
4. ALL BARS SHALL BE FREE OF LOOSE AND FLAKY RUST AND SCALE, OR OTHER MATERIALS WHICH MIGHT AFFECT THE ADHESION.
5. RELATED REIN. MESH (WALL) SHALL CONFORM TO ASTM A18-1M, EXCEPT AT SABS ON GRADE WHICH MAY BE GR60, LVS 60S W/ID 12" AND 12" MIN. LONG.

STRUCTURAL STEEL

1. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:
 BASE PLATES
 ANCHOR BOLTS
 ASTM A36
 ASTM F563, SS304/316
2. IF MATERIAL DOES NOT CONFORM WITH THE ASTM STANDARDS LISTED IN THE STRUCTURAL DRAWINGS, MATERIAL TEST REPORTS OR REPORTS OF TESTS MADE BY THE FABRICATOR OR A TESTING LABORATORY SHALL CONSTITUTE SUFFICIENT EVIDENCE OF CONFORMITY WITH THE DESIGNATED ASTM STANDARDS LISTED IN AISC 360 SECTION A3.
3. ALL STRUCTURAL STEEL SHALL CONFORM TO AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. BOLT HOLE SHALL BE 1/16" OVERSIZED, UNLESS NOTED OTHERWISE IN DRAWINGS. NOTIFY OR IF LARGER HOLES ARE NEEDED.

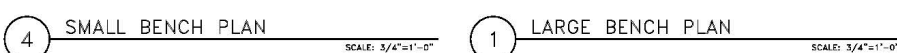
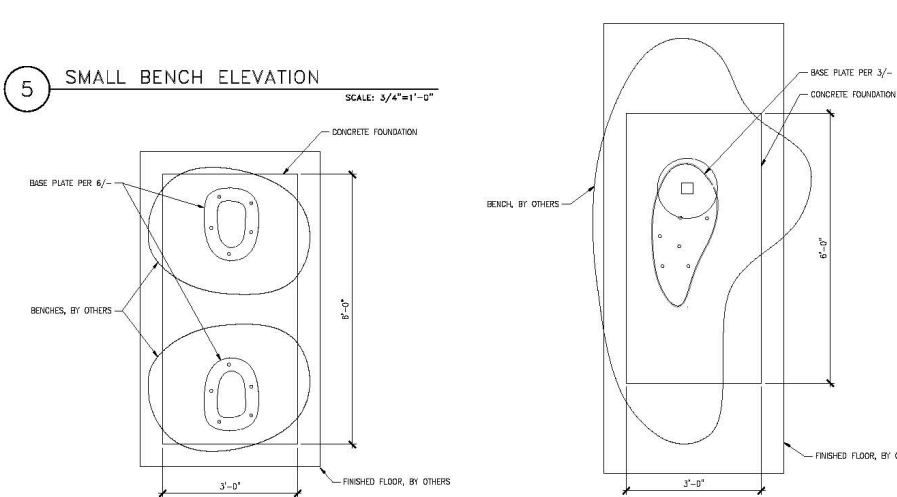
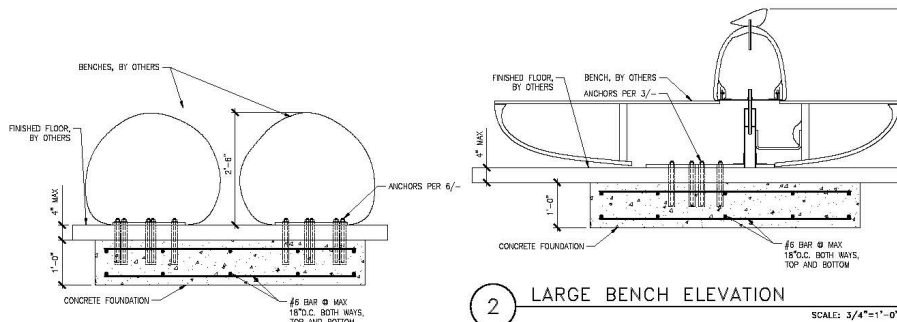
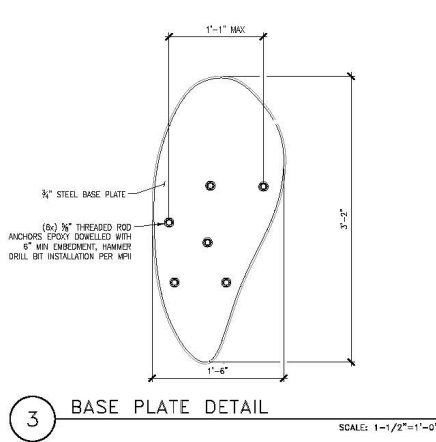
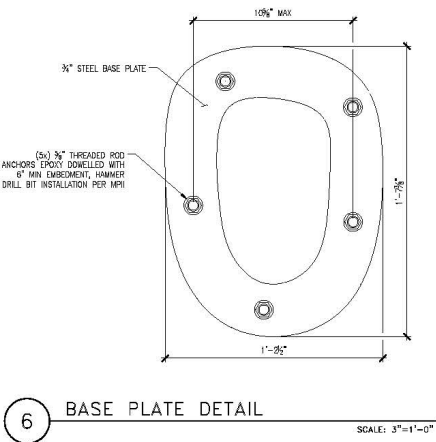
SPECIAL INSPECTIONS

1. AS SPECIFIED BY CBC CHAPTER 17, SPECIAL INSPECTIONS AND TESTS ARE NOT REQUIRED FOR CONSTRUCTION OF A MINOR NATURE IF APPROVED BY THE BUILDING OFFICIAL. IF SPECIAL INSPECTIONS ARE REQUIRED BY THE BUILDING DEPARTMENT, THE FOLLOWING CONTINUOUS OR PERIODIC SPECIAL INSPECTIONS SHALL BE PERFORMED BY THE OWNER'S TESTING AND INSPECTION AGENCY:
- a. CONCRETE. DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
 - b. EPOXY CONNECTED HOLDOWNS TO EXISTING CONCRETE. SEE EPOXY ANCHORING SECTION OF THE GENERAL NOTES FOR ADDITIONAL INFORMATION.

EPOXY ANCHORING:

1. EPOXY OR RESIN ANCHOR SHALL BE ON OF THE FOLLOWING:
 - * HULT-HIT-200 VS ADHESIVE (CC-15-ESB-4884)
 - * EQUIVALENT ALTERNATES WILL BE CONSIDERED UPON REQUEST AND SUBMISSION OF SPECIFICATIONS AND ICBO EVALUATION REPORT.
2. EPOXY ANCHORS WILL BE USED IN ALL LOCATIONS WHERE EITHER THREADED STEEL ROD OR REBAR IS BEING EMBEDDED INTO EXISTING CONCRETE OR MASONRY AS TENSION AND/OR SHEAR ELEMENTS.
3. CONTRACTOR SHALL MIX AND INSTALL RESIN AND HARDENER, DRILL AND PREPARE HOLES PER MANUFACTURER'S SPECIFICATION.
4. EXISTING REINFORCEMENT SHALL NOT BE CUT OR DAMAGED UNLESS PERMITTED IN WRITING BY THE ENGINEER.
5. CONTINUOUS OR PERIODIC SPECIAL INSPECTION IS REQUIRED FOR THREADED STEEL ROD EPOXY SET IN EXISTING CONCRETE OR MASONRY.
6. CONTINUOUS SPECIAL INSPECTION SHALL BE THE FULL THE OBSERVATION OF WORK BY AN EPCO. CONTINUOUS SPECIAL INSPECTION WILL BE PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. INSPECTOR SHALL OBSERVE WORK IN ACCORDANCE TO THE APPROVE DRAWINGS AND NOTIFY ENGINEER OF ANY EXCEPTIONS.
7. PERIODIC SPECIAL INSPECTION SHALL BE AS FOLLOWS:
 - * SET OF TENSION BOLTS SHALL BE MINIMUM OF 2 ANCHORS SHALL BE TESTED USING THE DIRECT TENSION LOAD METHOD.
 - * TENSION RODS SHALL BE SUBJECT TO DIRECT TENSION LOAD AS SHOWN FOR FIVE MINUTES (FIVE MINUTES) THE DISTANCE BETWEEN THE ANCHOR AND THE TEST APPARATUS SUPPORT SHALL NOT BE LESS THAN 75 PERCENT OF THE EMBEDMENT OF THE ANCHOR BOLT.
 - * 20% OF ANCHOR BOLTS TORQUE TEST

TESTING LOAD, U.O.N.		
THREADED ROD DIAMETER	DIRECT TENSION LOAD	TORQUE TEST LOAD
5/8" ϕ	1750 LBS	50 FT-LB



MASAKO MIKI BENCHES

Minna St. and New Montgomery St.
San Francisco, California 94105

[illegible]

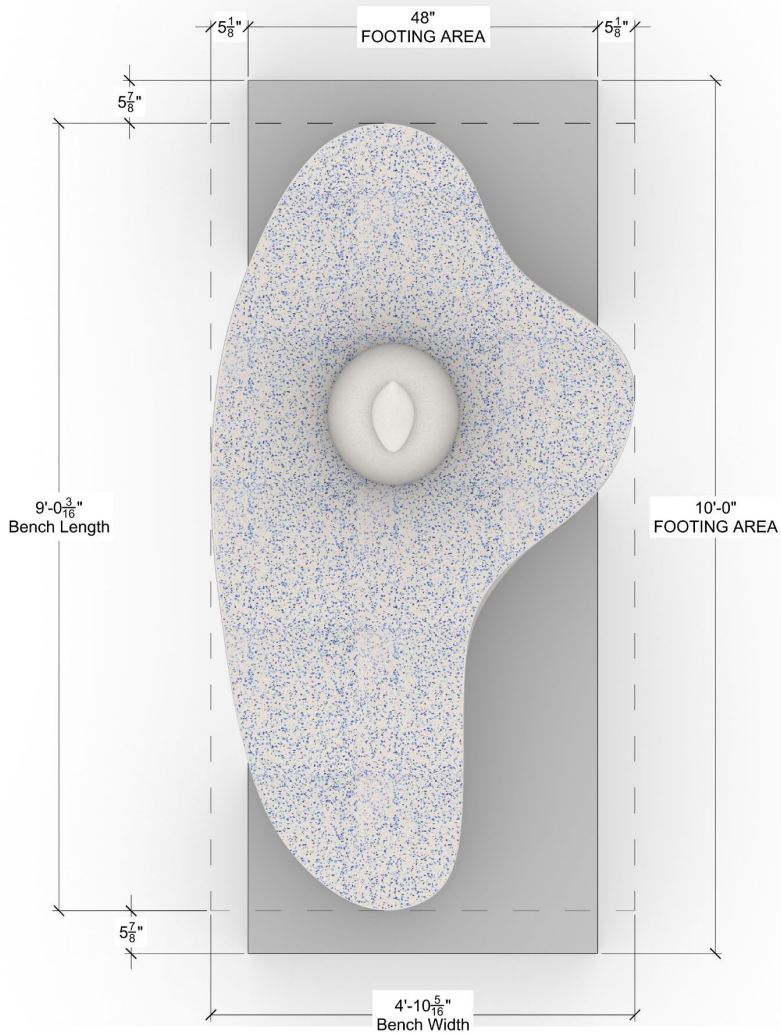
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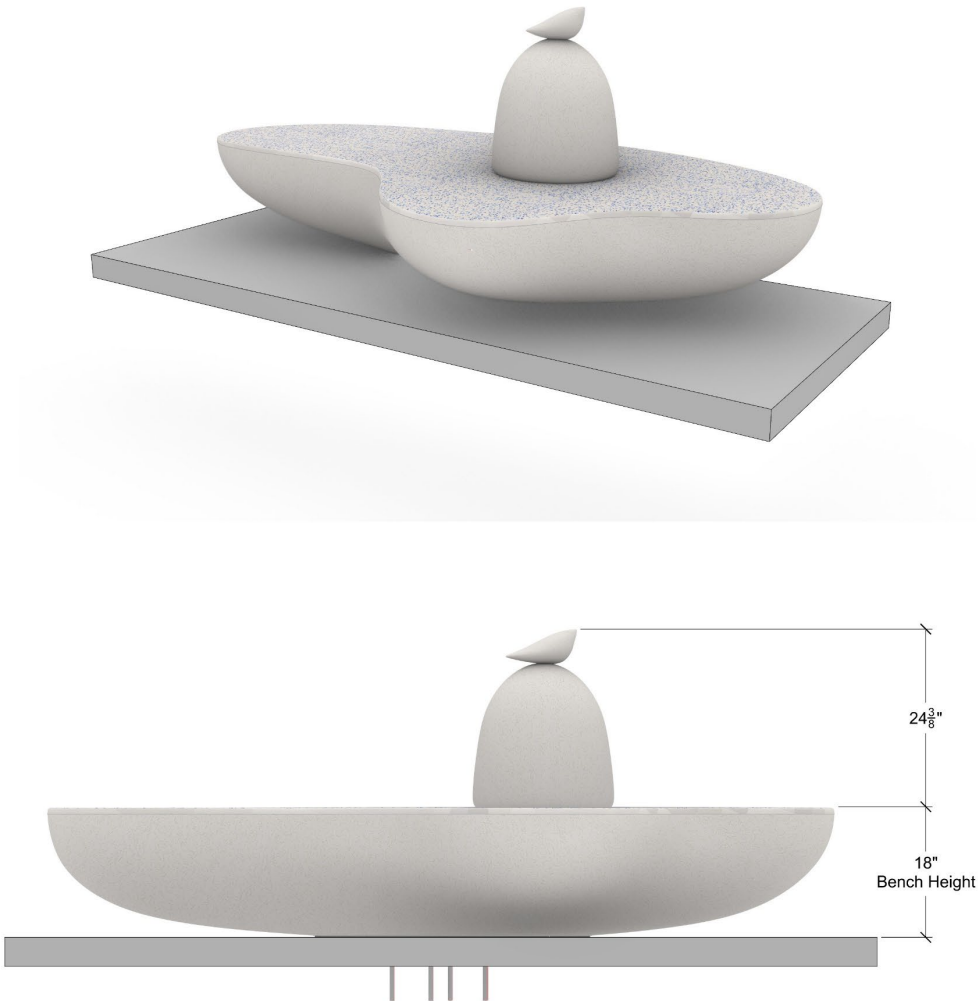
GENERAL NOTES
AND ELEVATION

SHEET

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1 **GENERAL DIMENSIONS - Large Bench**
SCALE: NA



Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

NO.	DESCRIPTION	DATE
0	R1	2026 06 05

Production Signoffs:
STRUCTURAL:

PM:

QA:

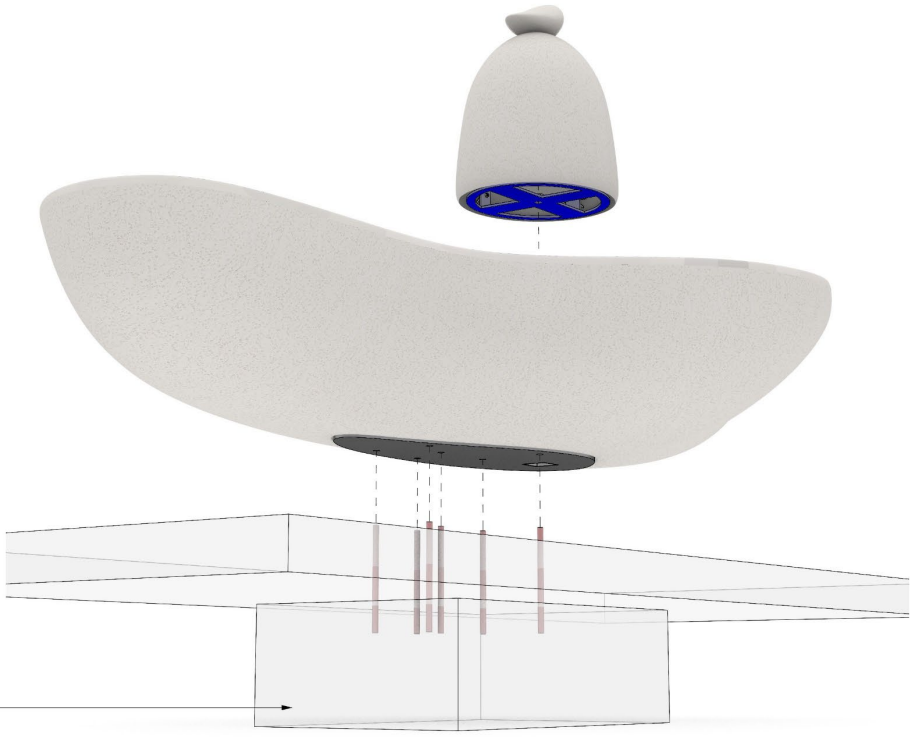
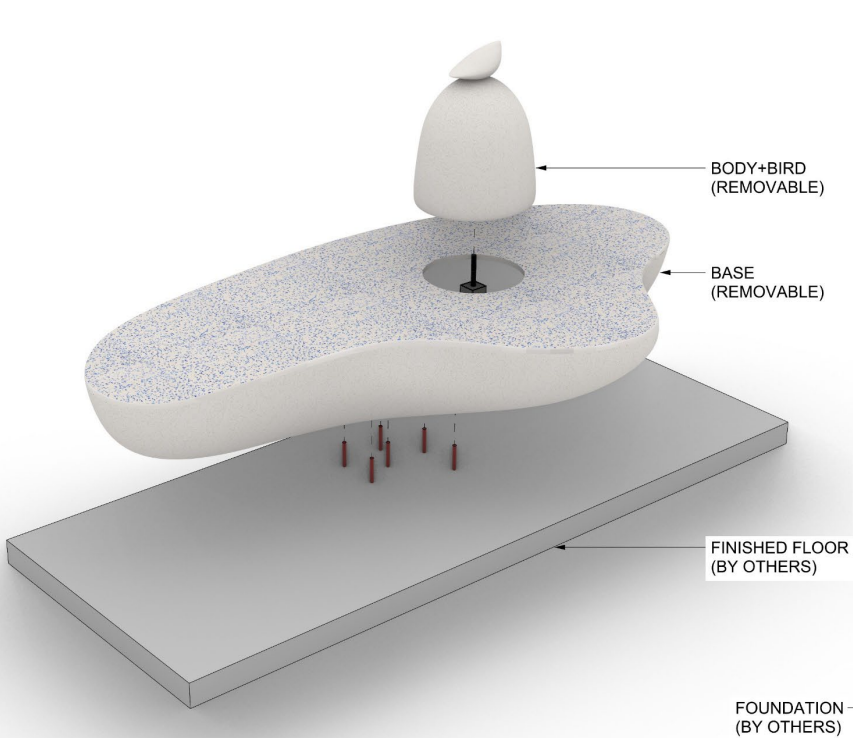
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**GENERAL
DIMENSIONS -
Large Bench**

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Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

NO.	DESCRIPTION	DATE
0	R1	2026 06 05



Production Signoffs:
STRUCTURAL:

PM:

QA:

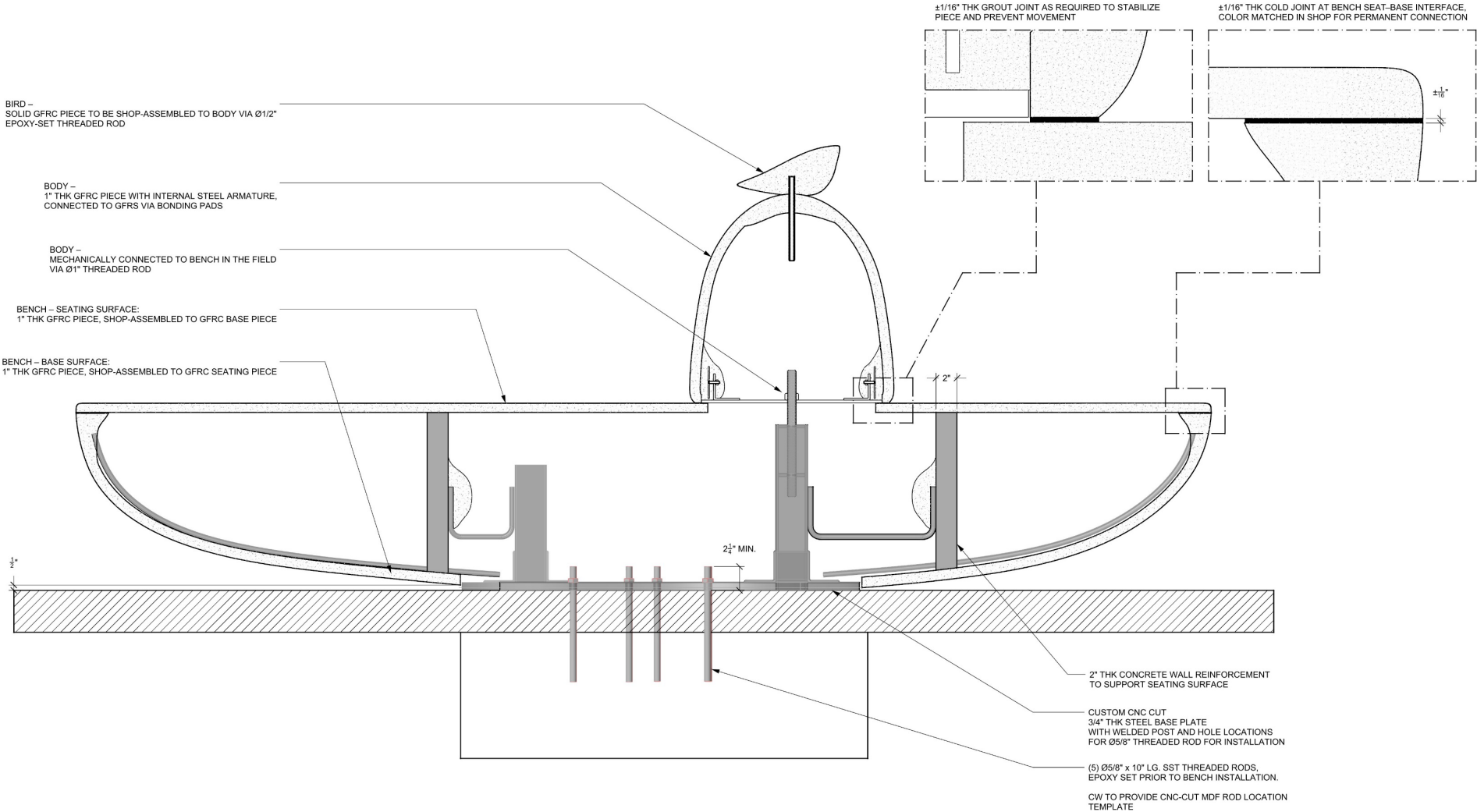
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**EXPLODED
ASSEMBLY -
Large Bench**

Sheet No.:
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1 **Shipped to Site as Separate Base and Body Pieces**
SCALE: NA

NOTE:
TOTAL WEIGHT ALL PARTS ~1000 LBS ±15%



1 **SECTION - Large Bench**
SCALE: NA

Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

NO.	DESCRIPTION	DATE
0	R1	2026 06 05

Production Signoffs:
STRUCTURAL:

PM:

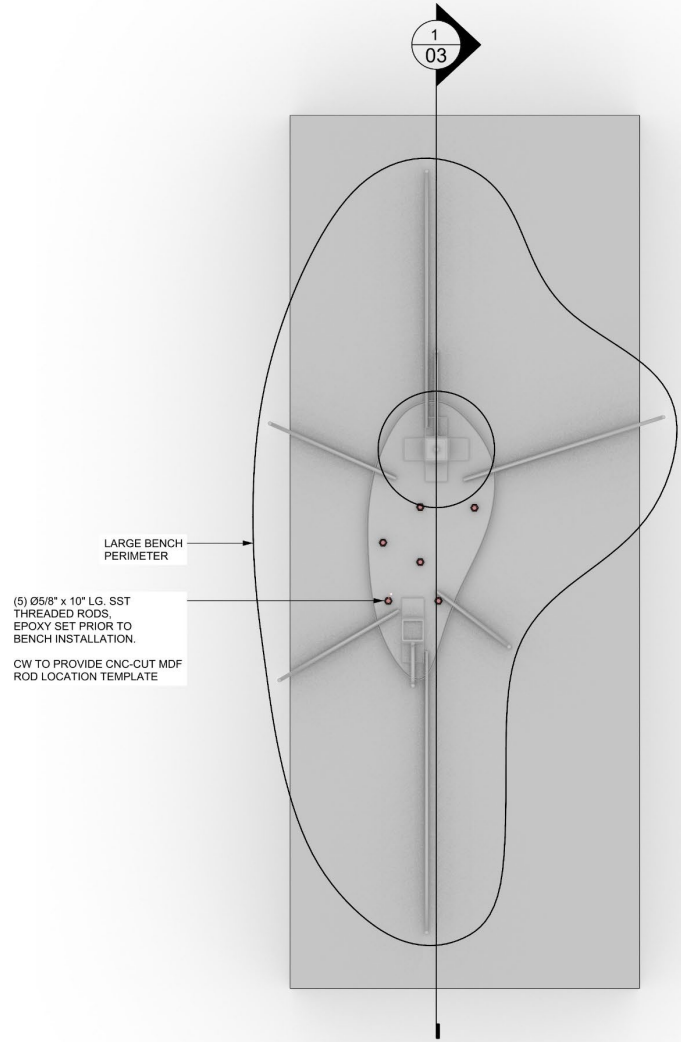
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**SECTION -
Large Bench**

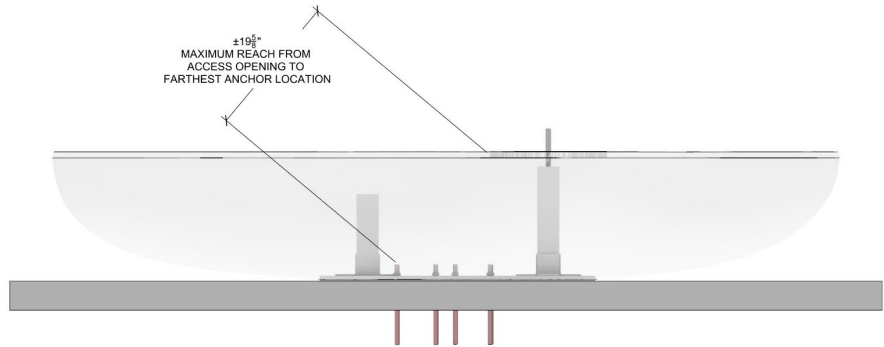
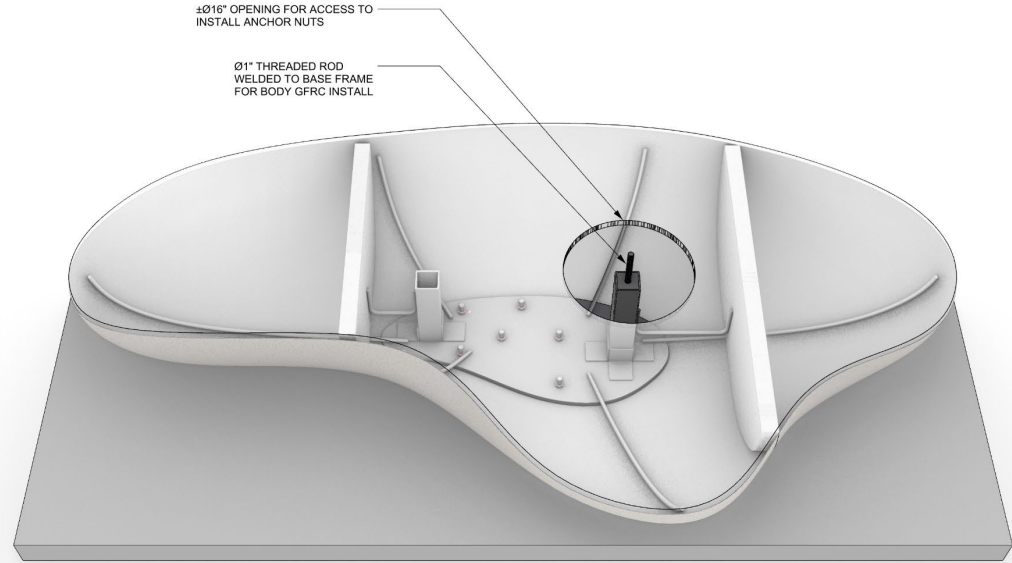
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1 **BASE PLATE INSTALL - Large Bench**
SCALE: NA



Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

NO.	DESCRIPTION	DATE
0	R1	2026 06 05

Production Signoffs:
STRUCTURAL:

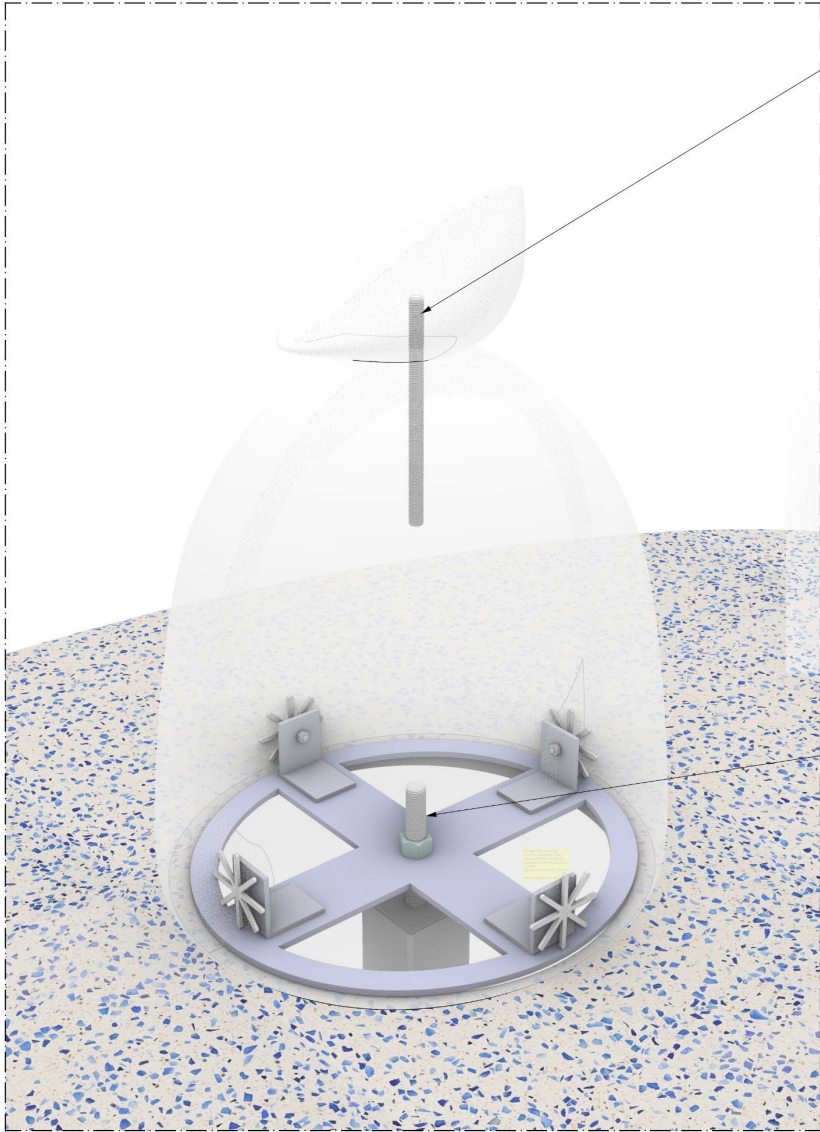
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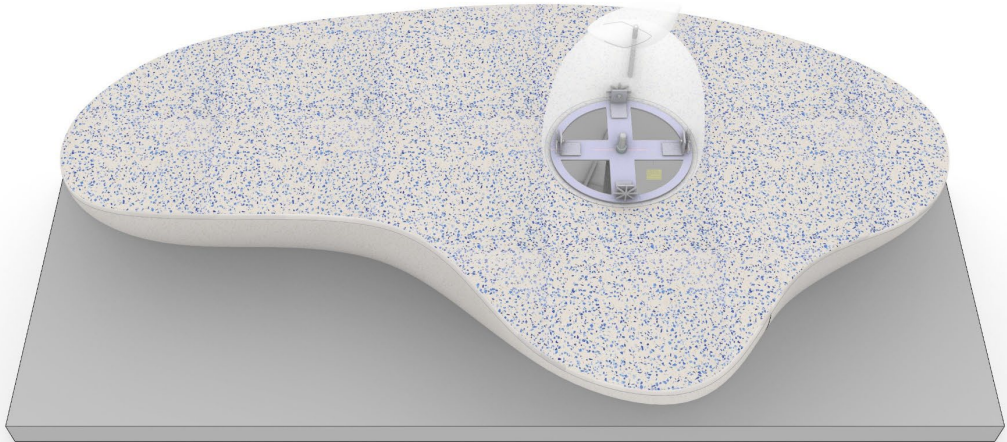
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**BASE PLATE
INSTALL -
Large Bench**

Sheet No.:
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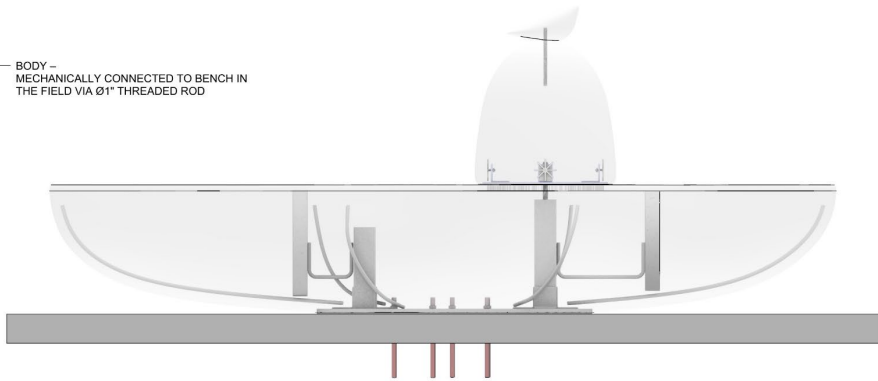
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BIRD –
SOLID GFRC PIECE TO BE SHOP-ASSEMBLED
TO BODY VIA Ø1/2" EPOXY-SET THREADED ROD



BODY –
MECHANICALLY CONNECTED TO BENCH IN
THE FIELD VIA Ø1" THREADED ROD



1 **BIRD+BODY INSTALL - Large Bench**
SCALE: NA

Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

NO.	DESCRIPTION	DATE
0	R1	2026 06 05

Production Signoffs:
STRUCTURAL:

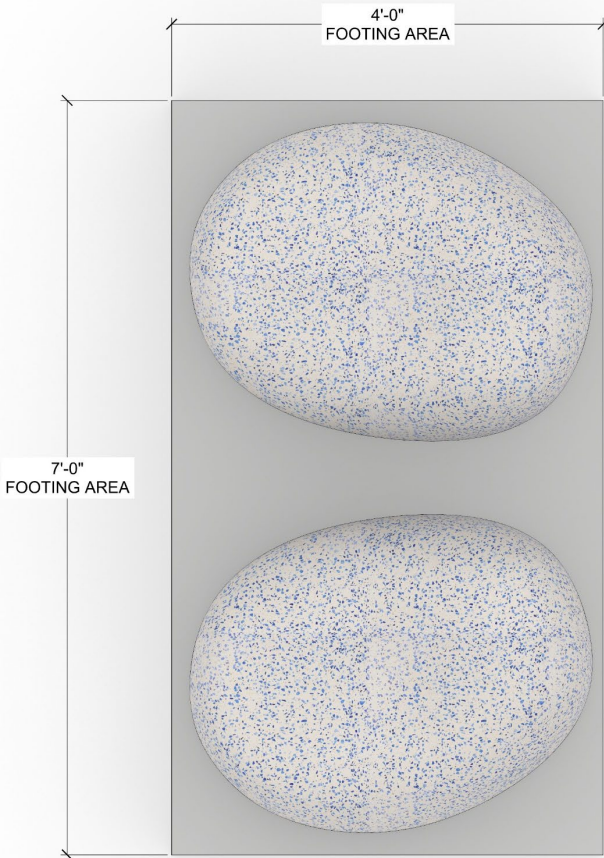
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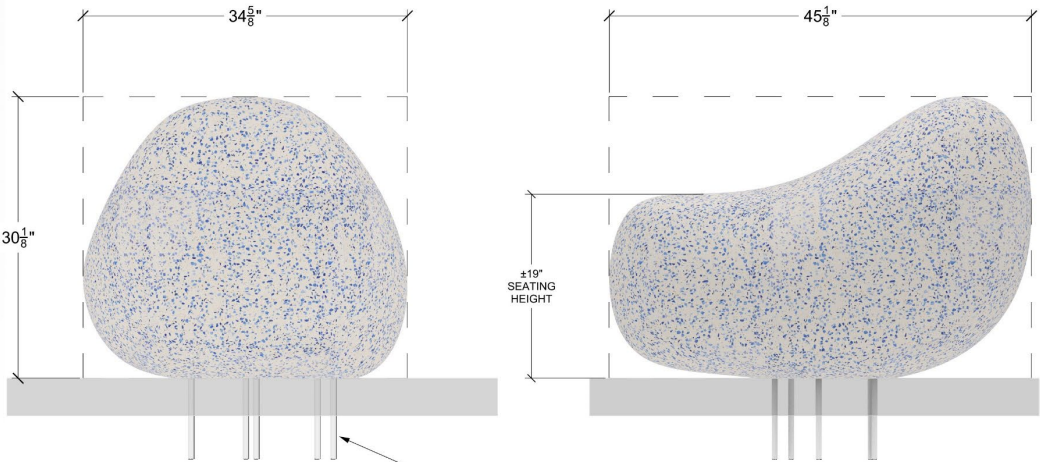
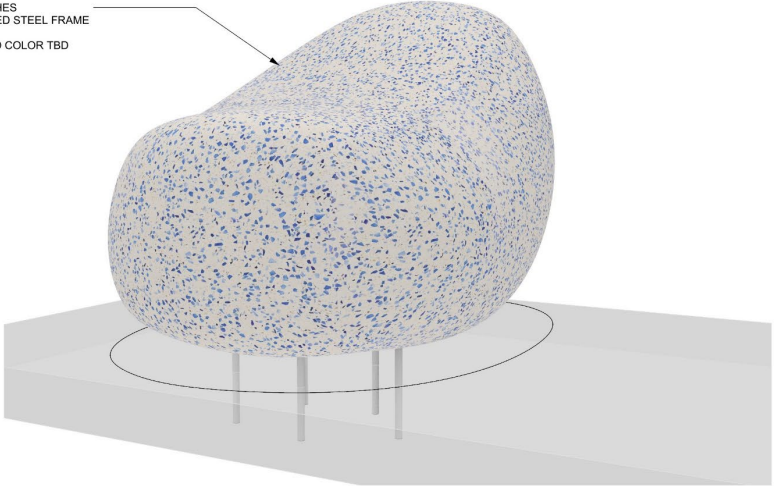
Sheet Name:
**BIRD+BODY
INSTALL -
Large Bench**

Sheet No.:
05

NOTE:
- SMALL BENCH LOCATIONS AND ORIENTATION TO BE COORDINATED IN FIELD.
- TERRAZZO COLOR AND AGGREGATE SPEC TBD
- TOTAL WEIGHT PER BENCH ~400 LBS ±15%



(2) SMALL BENCHES
WITH INTEGRATED STEEL FRAME
FINAL TERRAZZO COLOR TBD



(5) Ø5/8" x 10" LG. SST THREADED RODS,
EPOXY SET PRIOR TO BENCH INSTALLATION.
CW TO PROVIDE CNC-CUT MDF ROD LOCATION
TEMPLATE

1 **GENERAL DIMENSIONS - Small Bench**
SCALE: NA

Project Information:
Minna Natoma
Minna St at New
Montgomery St

Issued For:

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0	R1	2026 06 05

Production Signoffs:
STRUCTURAL:

PM:

QA:

Sheet Name:
**GENERAL
DIMENSIONS -
Small Bench**

Project Information:
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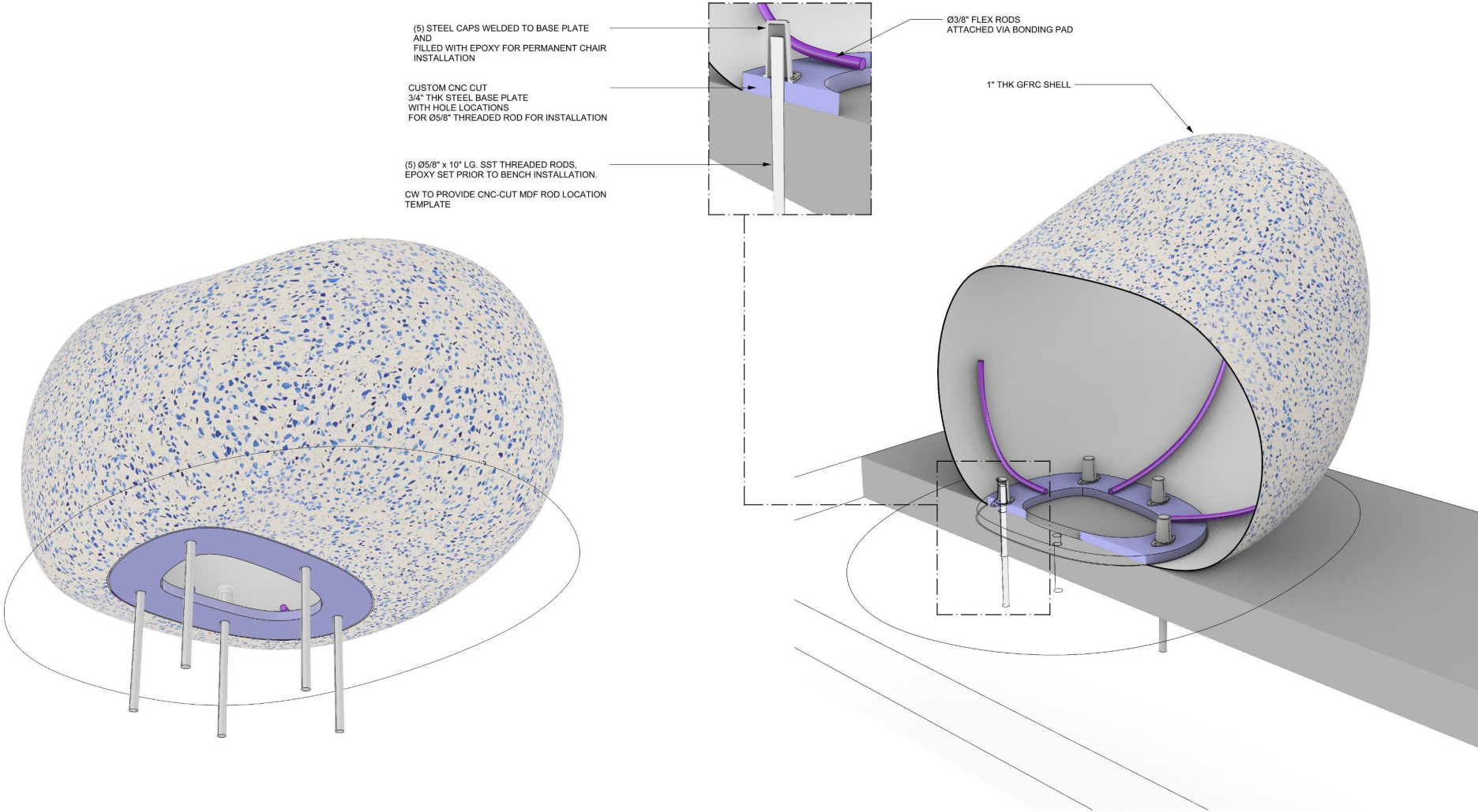
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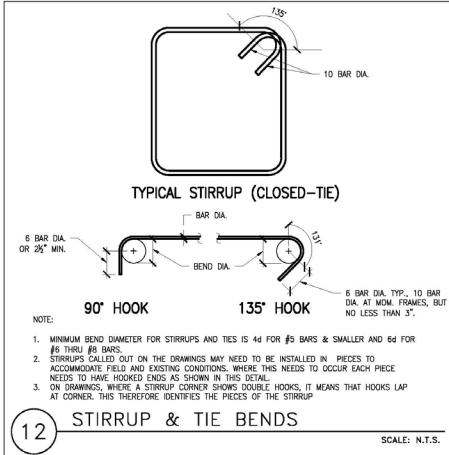
Sheet Name:
**BASE PLATE
INSTALL -
Small Bench**

Sheet No.:
07



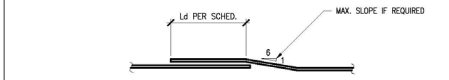
1 **BASE PLATE INSTALL - Small Bench**
SCALE: NA

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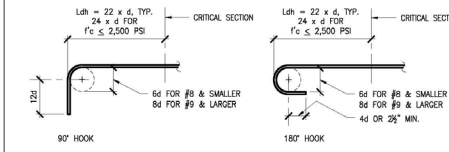


12 STIRRUP & TIE BENDS SCALE: N.T.S.

DEVELOPMENT LENGTH AND LAP SPICE (Ld) SCHEDULE (INCHES) CLASS B						
SIZE	LOCATION	CONCRETE COMPRESSIVE STRENGTH (PSI)				
		2,500	3,000	4,000	5,000	6,000
#4	HORIZ. TOP BAR	41	38	33	29	27
	ALL OTHER BARS	32	29	26	23	21
#5	HORIZ. TOP BAR	51	47	41	36	33
	ALL OTHER BARS	39	36	31	28	26
#6	HORIZ. TOP BAR	61	56	49	44	40
	ALL OTHER BARS	47	43	37	34	31
#7	HORIZ. TOP BAR	82	81	71	63	58
	ALL OTHER BARS	69	63	54	49	45
#8	HORIZ. TOP BAR	102	93	81	72	66
	ALL OTHER BARS	78	72	62	56	51
#9	HORIZ. TOP BAR	115	106	91	81	74
	ALL OTHER BARS	88	81	70	63	57
#10	HORIZ. TOP BAR	129	118	102	92	84
	ALL OTHER BARS	100	91	79	71	64
#11	HORIZ. TOP BAR	143	131	114	102	93
	ALL OTHER BARS	110	101	87	78	71



B: LAP SPICE



A: STANDARD HOOK

- NOTES:
1. d = REBAR DIAMETER
 2. LAP SPICE LENGTHS ARE BASED ON 60 KSI REBAR YIELD STRENGTH AND NORMAL CONCRETE WEIGHT
 3. TOP BAR IS A HORIZONTAL BAR (OTHER THAN IN WALLS) PLACED WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW BARS
 4. LAP SPICE LENGTHS ARE BASED ON MINIMUM CLEAR COVER GREATER THAN ONE BAR DIAMETER AND MINIMUM CLEAR SPACING GREATER THAN TWO BAR DIAMETERS
 5. IF EITHER REQUIREMENT IN NOTE 4 IS NOT SATISFIED, INCREASE LAP SPICE LENGTH BY 50%.

- GENERAL NOTES
1. ALL WORK AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2025 CALIFORNIA BUILDING CODE (REFERRED TO HEREINAFTER AS "CBC") AND 2025 SAN FRANCISCO BUILDING CODE AMENDMENTS.
 2. ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE. UNLESS NOTED OTHERWISE, NOTES AND DETAILS ON THE DRAWINGS TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS.
 3. ALL OMISSIONS AND CONFLICTS BETWEEN VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR ARCHITECTURAL SPECIFICATIONS (WHERE APPLICABLE) SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY OF THE WORK INVOLVED.
 4. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE INCLUDING SAFETY OF THE PERSONS AND PROPERTY, AND FOR ALL NECESSARY INDEPENDENT ENGINEERING REVIEWS OF THESE CONDITIONS. THE ARCHITECT'S OR ENGINEER'S JOB SITE REVIEW IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.
 5. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
 6. IN NO CASE SHALL WORKING DIMENSIONS BE SOLELY FROM PLANS, SECTIONS OR DETAILS ON THE STRUCTURAL DRAWINGS.
 7. NO OPENINGS, CHASES, NOTCHES, ETC. SHALL BE PLACED IN STRUCTURAL ELEMENTS UNLESS SPECIFICALLY NOTED ON THESE DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW SUCH OPENINGS.
 8. CONTRACTOR SHALL COORDINATE ALL STRUCTURAL FRAMING WITH MECHANICAL, PLUMBING AND ELECTRICAL INFRASTRUCTURE, INCLUDING, BUT NOT LIMITED TO, REDUCED AND SEMI-REDUCED FLOORING, MECHANICAL DUCTS AND PIPING, THE SPRINKLER PIPE AND HEADS AND PLUMBING DRAINS, WASTE AND SUPPLY LINES.
 9. ALL ASTM DESIGNATIONS SHALL BE AS AMENDED TO DATE UNLESS NOTED OTHERWISE.
- DESIGN CRITERIA
1. DEAD LOADS:
 - a. NON-STRUCTURAL COMPONENTS (TYPICAL) = 200-4000 LB
 - b. MISCELLANEOUS LOADS (TYPICAL) = 100-300 PSF
 2. LIVE LOADS:
 - a. CONCENTRATED LATERAL LOADS = 100-200 LB
 - b. DISTRIBUTED GRAVITY LOADS = 100-250 PSF
 3. SEISMIC DESIGN PARAMETERS:
 - a. IMPORTANCE FACTOR I = 1.0
 - b. RISK CATEGORY II
 - c. SITE CLASS D
 - d. MAPPED SHORT PERIOD ACCELERATION $a_s = 1.50$
 - e. SITE COEFFICIENT $S_{DS} = 1.12$
 - f. MAPPED ONE SECOND ACCELERATION $a_1 = 0.60$
 - g. SITE COEFFICIENT $S_{D1} = 1.5$
 - h. DESIGN ONE SECOND ACCELERATION $S_{D1} = 1.13$
 - i. SEISMIC DESIGN CATEGORY D
 4. DESIGN BASE SHEAR: $V = C_w W$ AT STRENGTH LEVEL (W = EFFECTIVE SEISMIC WEIGHT)
 5. WIND DESIGN PARAMETERS:
 - a. BASIC WIND SPEED 110mph
 - b. RISK CATEGORY II
 - c. EXPOSURE CATEGORY B
 - d. WIND PRESSURES (STEVEN'S LEVEL):
 - i. MAIN WIND FORCE RESISTING SYSTEM: 20 PSF
 - ii. COMPONENTS AND CLADDING: 30 PSF $(V_{E1} \leq -4")$
 6. FOUNDATION DESIGN PARAMETERS:
 - a. SPREAD FOOTING PARAMETERS:
 - i. ALLOWABLE SOIL PRESSURE: 1,500 PSF
 - ii. DEAD PLUS LIVE PLUS SEISMIC: 1,995 PSF
 - iii. PASSIVE PRESSURE: 250 PSF
 - iv. COEFFICIENT OF FRICTION 0.30

- FOUNDATION
1. FOUNDATION DESIGN IS BASED ON THE PRESUMPTIVE LOAD BEARING VALUES OF SOIL GIVEN IN THE CALIFORNIA BUILDING CODE 2025, TABLE 1806.2.
 2. INSTALLATION OF THE FOUNDATION FOOTINGS OR PIERS WITH RESPECT TO THE DEPTH BELOW FINISHED OR NATURAL GRADE SHALL BE AT A MINIMUM ACCORDING TO THE FOUNDATION DETAILS ON THESE PLANS. FIELD DISCOVERED CONDITIONS MAY NECESSITATE DEEPER FOUNDATIONS.
 3. EXCEPT WHERE OTHERWISE SHOWN, EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE.
 4. ALL EXCAVATIONS AND SUBGRADE PREPARATION SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER PER THE GEOTECHNICAL REPORT (W/O REPORT USE PER CODE MINIMUM STANDARDS).
 5. ALL WATER, SOIL, AND OTHER DEBRIS SHALL BE REMOVED FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING OF CONCRETE.
 6. ALL BACKFILL WITH ENGINEERED FILLS SHALL BE COMPACTED TO 98% RELATIVE DENSITY, UNLESS OTHERWISE SPECIFIED IN THE GEOTECHNICAL REPORT (W/O REPORT USE PER CODE MINIMUM STANDARDS).

- REINFORCING BAR
1. REINFORCING STEEL SHALL BE DEFORMED BARS PER ASTM A615 WITH BAR MARKS LEGIBLY ROLLED INTO THE SURFACE INDICATING SIZE, TYPE, OF STEEL, AND YIELD STRENGTH DESIGNATION:
 - a. #3 BARS AND SMALLER: GR60 40 OR GRADE 60
 - b. #4 BARS AND LARGER: GR60 60
 - c. ALL BARS TO BE WELDED: GR60 60
 2. REINFORCING SHALL HAVE A MINIMUM LAP IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS SHOWN ON THESE DRAWINGS. STAGGER SPICES WHENEVER POSSIBLE. VERTICAL WALL REINFORCING BARS SHALL EITHER EXTEND INTO FOOTINGS OR LAP SPLICED WITH FOOTING DOWELS OF THE SAME SIZE BARS.
 3. BENDING OF REINFORCING SHALL BE IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS SHOWN ON THESE DRAWINGS. FIELD BENDING OF BARS THAT ARE IN PLACE IS NOT PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
 4. ALL BARS SHALL BE FREE OF LOOSE AND PLANT RUST AND SCALE, GREASE, OR OTHER MATERIALS WHICH MIGHT AFFECT OR IMPAIR BOND.
 5. WELDED WIRE MESH (WWM) SHALL CONFORM TO ASTM A1064, EXCEPT AT SLABS ON GRADE WHICH MAY BE GR60. USE BAR W10/10 AND LAP 12" MIN. U.O.A.

- EPOXY ADHESIVE ANCHORS:
1. EPOXY ADHESIVE SHALL BE ONE OF THE FOLLOWING:
 - a. HILTI HIT-RE 500 ADHESIVE (HILTI REPORT 10-2332)
 - b. SIMPSON SET-30 ADHESIVE (ICC-ES REPORT ESR-2508)
 - c. EQUIVALENT ALTERNATIVES WILL BE CONSIDERED UPON REQUEST AND SUBMISSION OF MANUFACTURER'S SPECIFICATIONS AND ICC-ES REPORT.
 2. INSTALLATION OF EPOXY ADHESIVE ANCHORS HORIZONTALLY OR VERTICALLY INCISED TO SUPPORT TENSION TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE A208/CS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. THE ACCEPTABILITY OF CERTIFICATION OTHER THAN A208/CS ADHESIVE ANCHOR INSTALLER CERTIFICATION SHALL BE APPROVED BY THE ENGINEER OF RECORD.
 3. INSTALLATION REQUIREMENTS: INSTALL ADHESIVE AND ANCHORS PER MANUFACTURER'S SPECIFICATIONS. CONTRACTOR SHALL HAVE MANUFACTURER'S PRODUCT INSTALLATION LITERATURE AND PRODUCT EVALUATION REPORT ON SITE FOR REFERENCE DURING INSTALLATION.
 - a. ANCHORS SHALL BE INSTALLED IN CONCRETE THAT HAS A MINIMUM AGE OF 21 DAYS PER AC 308.5.
 - b. HOLE PREPARATION: HOLES SHALL BE DRILLED, BLOW OUT, AND BRUSHED PER MANUFACTURER'S SPECIFICATIONS.
 - c. CARTRIDGE PREPARATION: EPOXY ADHESIVE SHALL BE MIXED AND DISPENSED PER MANUFACTURER'S SPECIFICATIONS.
 4. VISUAL PERIODIC SPECIAL INSPECTION IS REQUIRED DURING INSTALLATION, AND SHALL BE PERFORMED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AND ICC-ES REPORT. THE SPECIAL INSPECTOR MUST BE ON THE JOB SITE TO VERIFY THE INITIAL INSTALLATIONS OF EACH TYPE AND SIZE OF ANCHOR BY CONSTRUCTION PERSONNEL. SUBSEQUENT INSTALLATIONS OF THE SAME TYPE AND SIZE BY THE SAME CONSTRUCTION PERSONNEL ARE PERMITTED TO BE PERFORMED IN THE ABSENCE OF THE SPECIAL INSPECTOR. MUST MAKE REGULAR INSPECTIONS AT INTERVALS DETERMINED BY THE ENGINEER OF RECORD. IF THE CONTRACTOR FAILS TO ENSURE VISUAL PERIODIC SPECIAL INSPECTION IS ADEQUATELY PERFORMED, OR THE LOCAL BUILDING AUTHORITY SPECIFICALLY REQUIRES TESTING, CONTACT THE ENGINEER FOR PROOF LOADING REQUIREMENTS.
 5. QUALITY CONTROL REQUIREMENTS:
 - a. SHEAR ANCHORS: 25% OF ANCHORS RESISTING SHEAR FORCES ONLY, SUCH AS SILL PLATE OR LEDGER ANCHORS, SHALL BE TORQUE TESTED TO THE VALUES LISTED BELOW.
 - b. TENSION ANCHORS: 18 (BUT NOT LESS THAN TWO) OF ANCHORS RESISTING TENSION FORCES, SUCH AS HOLDOWN ANCHORS, SHALL BE SUBJECT TO DIRECT TENSION TESTS PER THE TYPICAL TOLERANCES TO EXISTING CONCRETE DETAILS. AN ADDITIONAL 20% (BUT NOT LESS THAN THREE) SHALL BE TORQUE TESTED TO THE VALUES LISTED BELOW.
 - c. TORQUE TESTING REQUIREMENTS: TEST ANCHORS USING A TORQUE CALIBRATED WRENCH TO THE FOLLOWING MINIMUM TORQUE VALUES:

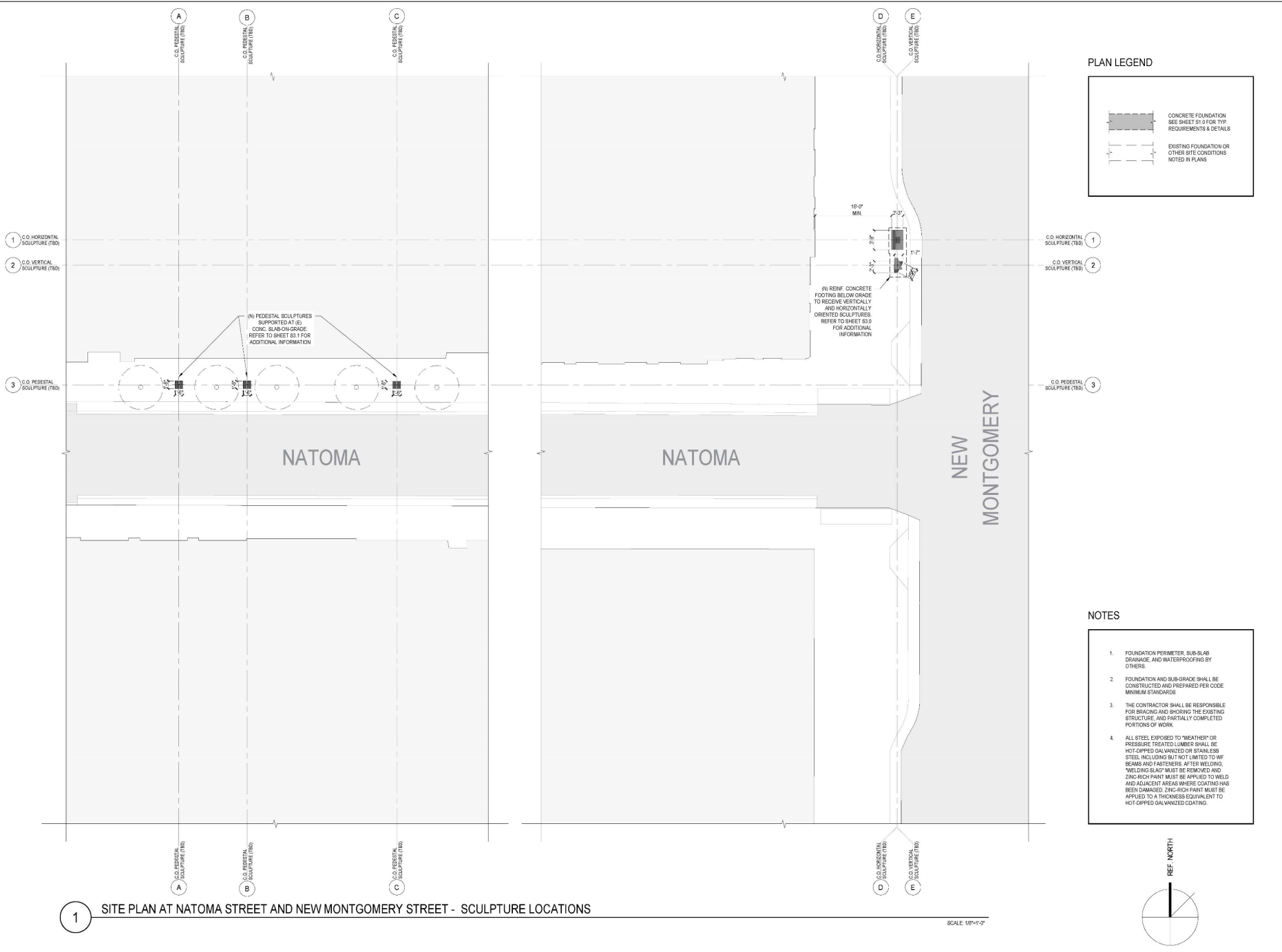
ANCHOR	40 FT.-LBS.
3/8"	50 FT.-LBS.
1/2"	60 FT.-LBS.
5/8"	70 FT.-LBS.
3/4"	80 FT.-LBS.

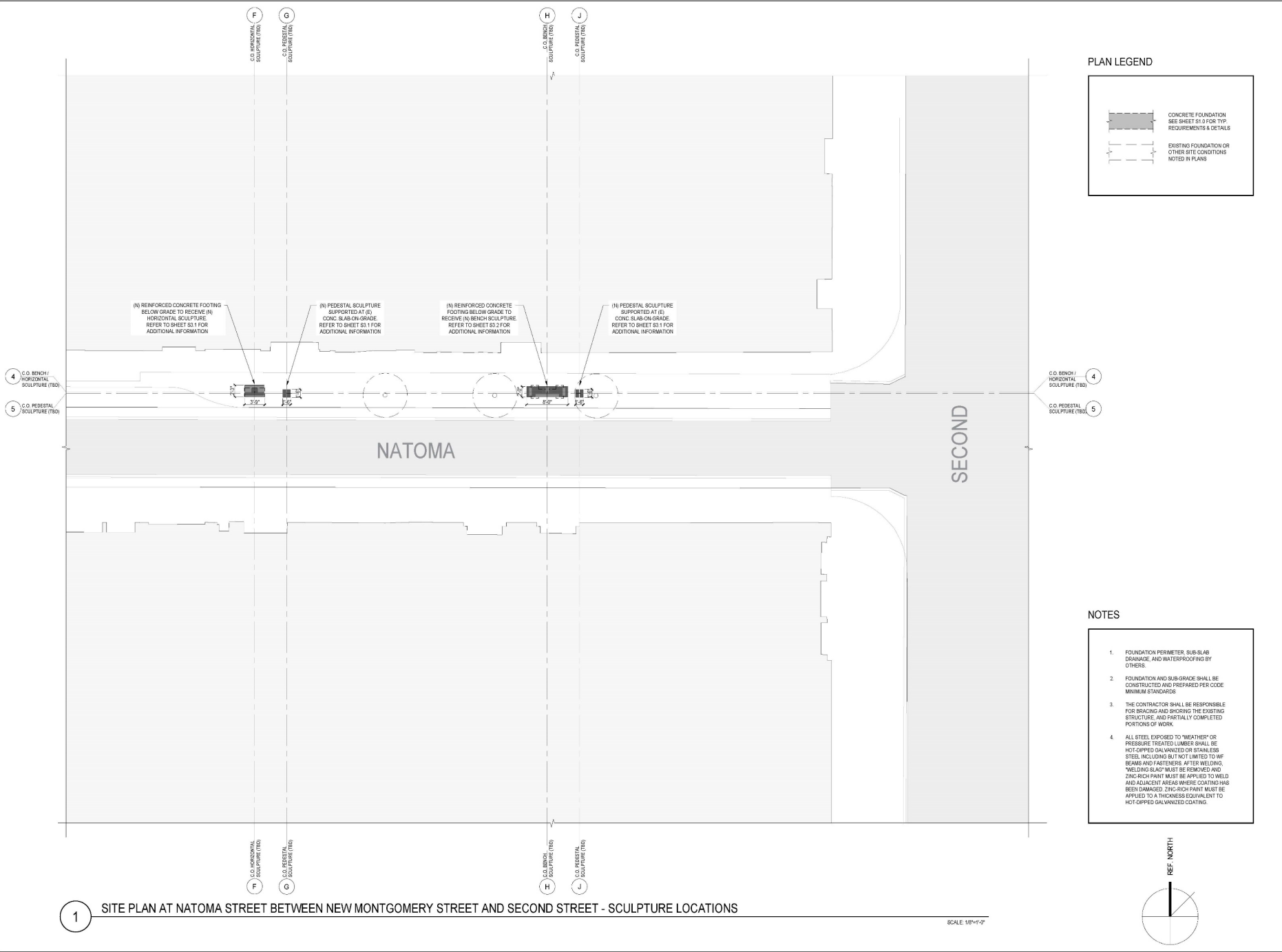
- DEMOLITION AND SHORING WORKS
1. ALL DESIGN AND DETAILING FOR TEMPORARY SHORING CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CALIFORNIA BUILDING CODE, 2025 EDITION. DRAWINGS AND CALCULATIONS SHALL BE STAMPED AND SIGNED BY A CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA, AND SHALL BE SUBMITTED TO THE LOCAL DEPARTMENT OF BUILDING INSPECTION FOR APPROVAL UPON REQUEST.
- STRUCTURAL OBSERVATIONS
1. THE FOLLOWING ITEMS SHALL HAVE PERIODIC STRUCTURAL OBSERVATION BY THE STRUCTURAL ENGINEER OF RECORD PER CBC SECTION 1704.6:
 - a. REINFORCING STEEL PRIOR TO POURING CONCRETE
 - b. ANCHOR BOLTS PRIOR TO POURING CONCRETE
 - c. STRUCTURAL FRAMING AND CONNECTORS PRIOR TO COVER
 2. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER A MINIMUM OF 48 HOURS (EXCLUDING WEEKEND DAYS) PRIOR TO THE TIME OF A REQUIRED STRUCTURAL OBSERVATION.
 3. OBSERVATION VISITS TO THE JOB SITE BY THE ENGINEER'S FIELD REPRESENTATIVE SHALL BE CONSIDERED AS NEITHER INSPECTION NOR APPROVAL OF CONSTRUCTION.

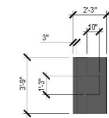
ABBREVIATIONS ON DRAWINGS:

A.B.	ANCHOR BOLT	LVZ.	MICROLAM
B.B.	BLOCKING	H.VZ.	HOT DIPPED GALVANIZED
BM	BEAM	(N)	NEW
CL	CENTERLINE	(O)	OVER
COL	COLUMN	O.C. OR O.C.C.	OPPOSITE HAND
CONC.	CONCRETE	PSL	PARALLEL
CONN.	CONNECTION	S.A.D.	SEE ARCHITECTURAL DRAWINGS
DA OR #	DIAMETER	S.W.	SHEAR WALL
D.S.	EXISTING	SW	SWALL
(E) OR EXIST.	ELEVATION	THD. ROD	THREADED ROD
EX.	EXISTING	TOP & BOTTOM	TOP & BOTTOM
F.A.	FACH FACE	TYP.	UNLESS OTHERWISE NOTED
E.N.	EDGE NAIL	VERT.	VERTICAL
F.A.	FACH FACE	V.F.T.	VERTICAL
F.F.	FACH FACE	W/F	WITH
FOTING	FOOTING		
GLM	GLU-LAM		

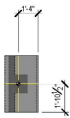




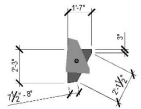




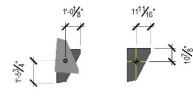
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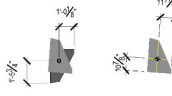
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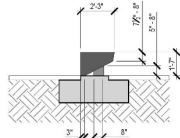
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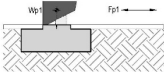
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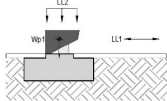
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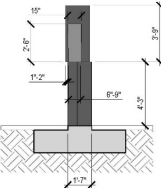
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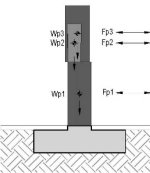
GRAVITY LOADS w/
SEISMIC LOADS



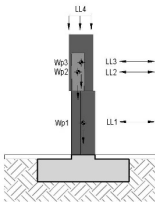
GRAVITY LOADS w/
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DIMENSIONS (SECTION)



GRAVITY LOADS w/
SEISMIC LOADS



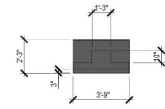
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DIRECTION 2

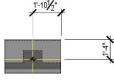
DIRECTION 2

4 NATOMA AND NEW MONTGOMERY STREETS - HORIZONTALLY ORIENTED SCULPTURE

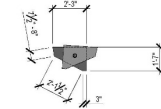
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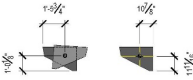
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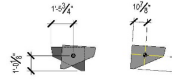
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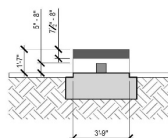
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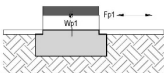
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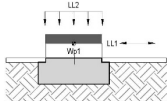
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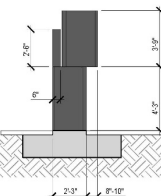
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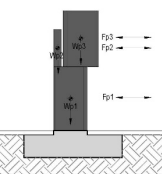
GRAVITY LOADS w/
SEISMIC LOADS



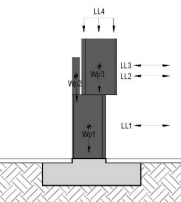
GRAVITY LOADS w/
LIVE LOADS



DIMENSIONS (SECTION)



GRAVITY LOADS w/
SEISMIC LOADS



GRAVITY LOADS w/
LIVE LOADS

DIRECTION 1

DIRECTION 1

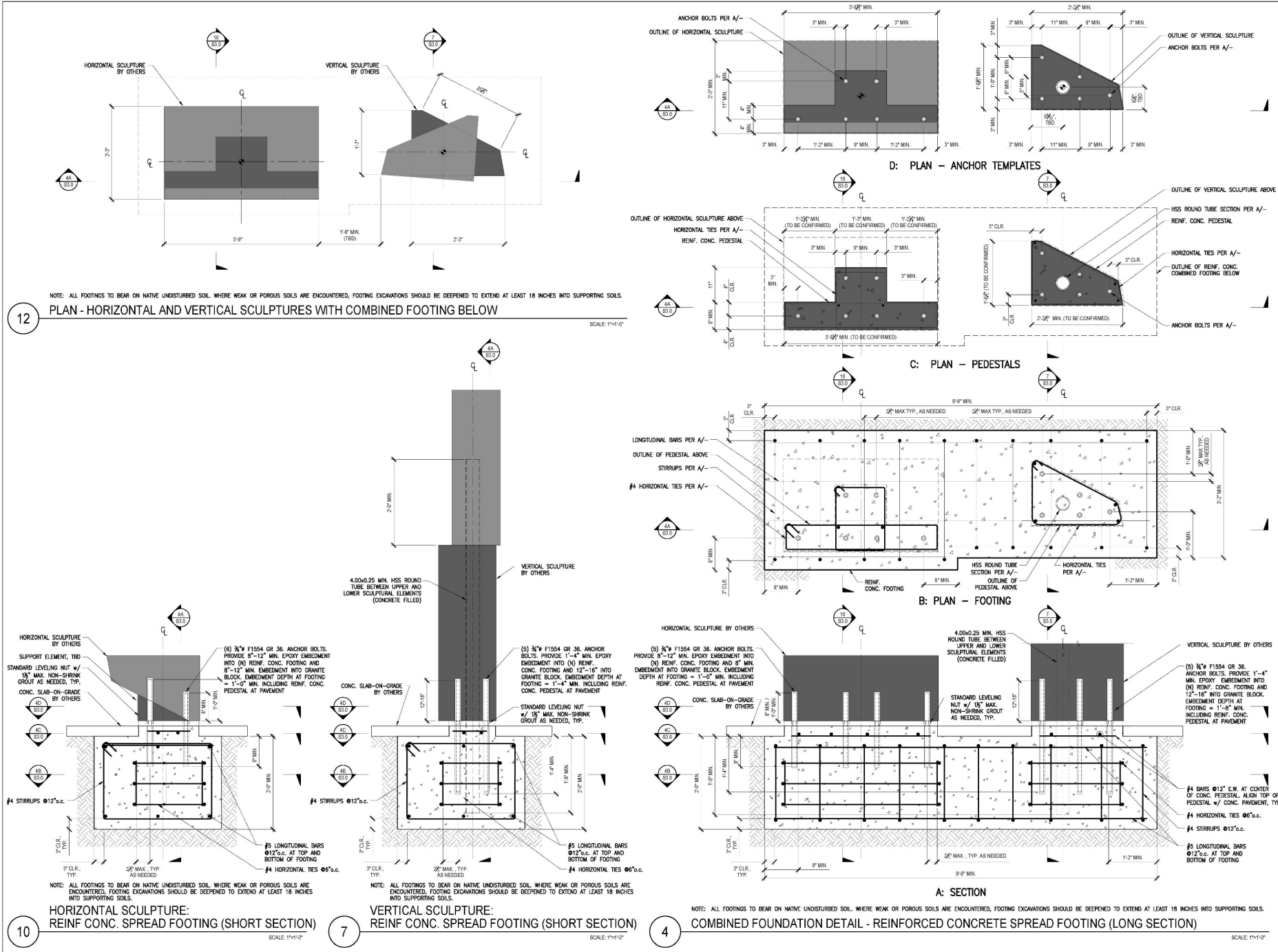
3 NATOMA AND NEW MONTGOMERY STREETS - HORIZONTALLY ORIENTED SCULPTURE

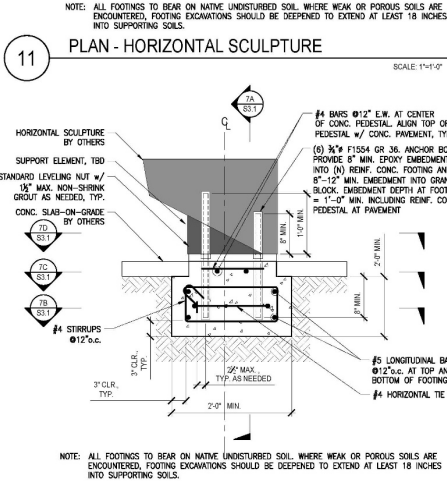
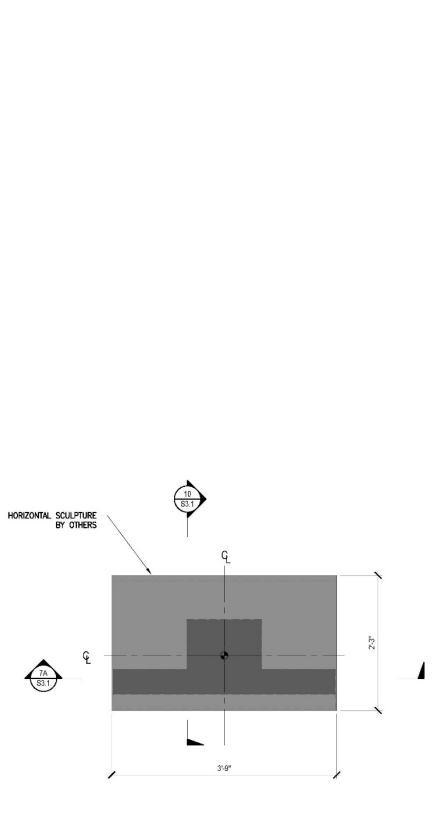
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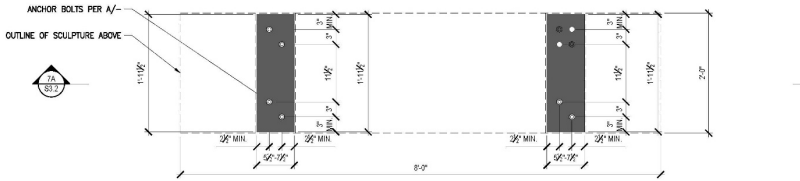
1 NATOMA AND NEW MONTGOMERY STREETS - VERTICALLY ORIENTED SCULPTURE

SCALE: 1/4"=1'-0"

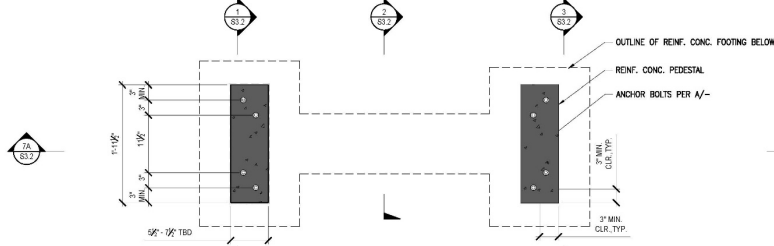




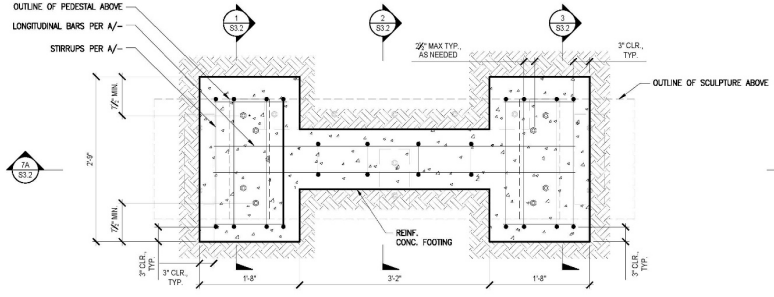




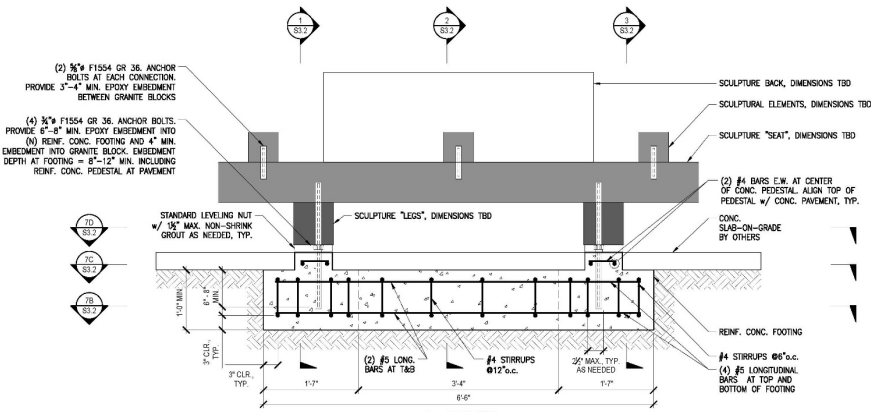
D: PLAN - ANCHOR TEMPLATES



C: PLAN - PEDESTALS



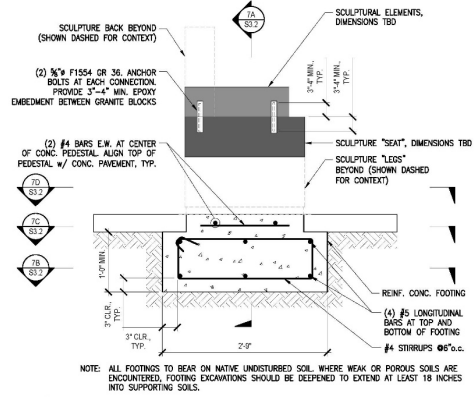
B: PLAN - FOOTING



A: SECTION

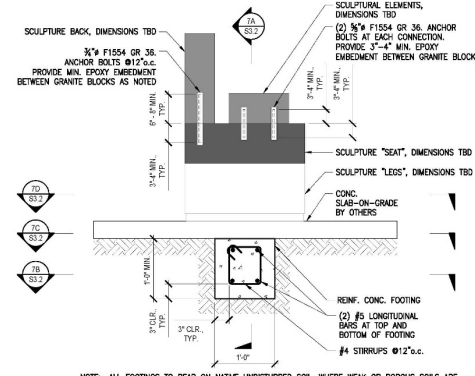
NOTE: ALL FOOTINGS TO BEAR ON NATIVE UNDISTURBED SOIL. WHERE WEAK OR POROUS SOILS ARE ENCOUNTERED, FOOTING EXCAVATIONS SHOULD BE DEEPENED TO EXTEND AT LEAST 18 INCHES INTO SUPPORTING SOILS.

FOUNDATION DETAIL - BENCH SCULPTURE REINFORCED CONCRETE SPREAD FOOTING (LONG SECTION)



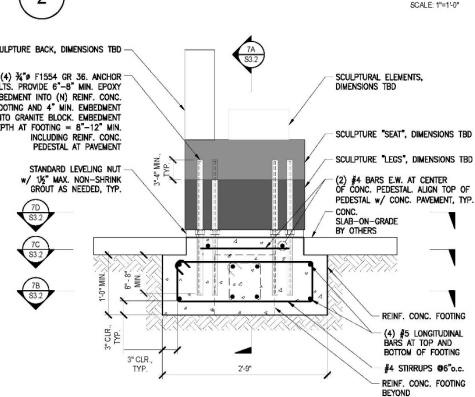
3 BENCH SCULPTURE - BACK AND CENTER ELEMENT

SCALE: 1"=1'-0"



2 BENCH SCULPTURE - SEAT AND EDGE ELEMENTS

SCALE: 1"=1'-0"



1 BENCH SCULPTURE - FOOTING (SHORT SECTION)

SCALE: 1"=1'-0"