## GENERAL NOTES

1. ALL WORK AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 CALIFORNIA BUILDING CODE (REFERRED TO HEREINAFTER AS "CBC"). AND 2022 SAN FRANCISCO BUILDING CODE (SFBC) AMENDMENTS.

2. ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE, UNLESS NOTED OTHERWISE. NOTES AND DETAILS ON THE DRAWINGS TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS.

3. ALL OMISSIONS AND CONFLICTS BETWEEN VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR ARCHITECTURAL SPECIFICATIONS (WHERE APPLICABLE) SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY OF THE WORK INVOLVED.

4. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE INCLUDING SAFETY OF THE PERSONS AND PROPERTY, AND FOR ALL NECESSARY INDEPENDENT ENGINEERING REVIEWS OF THESE CONDITIONS. THE ARCHITECT'S OR ENGINEER'S JOB SITE REVIEW IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.

5. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.

6. IN NO CASE SHALL WORKING DIMENSIONS BE SCALED FROM PLANS, SECTIONS OR DETAILS ON THE STRUCTURAL DRAWINGS.

7. CONTRACTOR SHALL FOLLOW AND COMPLY WITH ALL MANUFACTURER'S GUIDELINES AND SPECIFICATIONS OF THE PRODUCTS INCLUDED IN THE DRAWINGS.

8. ALL ASTM DESIGNATIONS SHALL BE AS AMENDED TO DATE UNLESS NOTED OTHERWISE.

9. IT IS SOLELY THE CLIENT'S RESPONSIBILITY TO ENSURE THAT THE U.S APPROVED MATERIALS LISTED IN THE GENERAL NOTES ARE USED AND THAT ANY SUBSTITUTES MEET THE APPROVED STANDARDS AND CRITERIA.

## DESIGN CRITERIA

1. DEAD LOADS:
   a. STRUCTURE SELF-WEIGHT: 300 LBS MAX (EACH)

2. LIVE LOADS:
   a. HORIZ. LIVE LOAD: 200 LBS
   b. VERTICAL LIVE LOAD ON JUMP ROPE: 200 LBS

3. SEISMIC DESIGN PARAMETERS:

| | | |
|---|---|---|
| a. | IMPORTANCE FACTOR | I = 1.0 |
| b. | RISK CATEGORY | II |
| c. | SITE CLASS | D |
| d. | MAPPED SHORT PERIOD ACCELERATION | $S_s$ = 1.5 |
| e. | SITE COEFFICIENT | $F_a$ = 1.20 |
| f. | DESIGN SHORT PERIOD ACCELERATION | $S_{DS}$ = 1.20 |
| g. | MAPPED ONE SECOND ACCELERATION | $S_1$ = 0.600 |
| h. | SITE COEFFICIENT | $F_v$ = 1.70 |
| i. | DESIGN ONE SECOND ACCELERATION | $S_{D1}$ = 0.680 |
| j. | SEISMIC DESIGN CATEGORY | D |

   DESIGN BASE SHEAR: V = Cs*W AT STRENGTH LEVEL
   (W = EFFECTIVE SEISMIC WEIGHT)

| | | |
|---|---|---|
| k. | STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE | $C_s$ = 0.400 |
| | RESPONSE MODIFICATION FACTOR | R = 3.0 |

4. WIND DESIGN PARAMETERS:

| | | |
|---|---|---|
| a. | BASIC WIND SPEED | 86mph |
| b. | RISK CATEGORY | II |
| c. | EXPOSURE CATEGORY | B |
| d. | WIND PRESSURES: | |
| | • MAIN WIND FORCE RESISTING SYSTEM: | 16.0psf |

5. FOUNDATION DESIGN PARAMETERS:

| | | |
|---|---|---|
| a. | SPREAD FOOTING PARAMETERS | |
| | • ALLOWABLE SOIL PRESSURE: | 1,500 PSF |

## FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT BY AGS, INC. DATED NOVEMBER 2021. A COPY OF THIS REPORT MAY BE OBTAINED FROM THE ARCHITECT'S OFFICE.

2. INSTALLATION OF THE FOUNDATION FOOTINGS OR PIERS WITH RESPECT TO THE DEPTH BELOW FINISHED OR NATURAL GRADE SHALL BE AT A MINIMUM ACCORDING TO THE FOUNDATION DETAILS ON THESE PLANS. FIELD DISCOVERED CONDITIONS MAY NECESSITATE DEEPER FOUNDATIONS.

3. EXCEPT WHERE OTHERWISE SHOWN, EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE.

4. ALL EXCAVATIONS ARE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.

5. ALL WATER, SOIL, AND OTHER DEBRIS SHALL BE REMOVED FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING OF CONCRETE.

6. ALL BACKFILL WITH ENGINEERED FILLS SHALL BE COMPACTED TO 95% RELATIVE DENSITY.

## CONCRETE

1. ALL CONCRETE CONSTRUCTION SHALL BE PER CBC CHAPTER 19 AND IN ACCORDANCE WITH ACI 318-19, SPECIFICATIONS FOR STRUCTURAL CONCRETE.

2. ALL CONCRETE SHALL HAVE A MAXIMUM WATER-CEMENT RATIO OF 0.48 FOR FOUNDATIONS AND ALL STRUCTURAL ELEMENTS AND 0.45 FOR SLABS, 4"±1" SLUMP, AND SHALL OBTAIN A 28 DAY MINIMUM COMPRESSIVE STRENGTH AS FOLLOWS:
   a. GRADE BEAMS, MAT SLABS, AND FOOTINGS 2,500 PSI

3. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE, WEIGHING LESS THAN 150 PCF, UNLESS OTHERWISE NOTED.

4. CEMENT SHALL CONFORM TO ASTM C150, TYPE II (OR ENGINEERED MAXIMUM DESIGN TO STRENGTH).

5. HARD ROCK AGGREGATES SHALL CONFORM TO ASTM C33. MAXIMUM NORMAL SIZE OF AGGREGATE SHALL NOT EXCEED 1 1/2 INCHES FOR FOUNDATION CONCRETE AND 1 INCH FOR STRUCTURAL CONCRETE ABOVE THE FOUNDATION. SEE ALSO THE REQUIREMENTS IN ACI STANDARD SPECIFICATIONS. MAXIMUM NORMAL SIZE SHALL ALSO BE SELECTED SUCH THAT WORKABILITY AND PLACEABILITY OF CONCRETE ARE FACILITATED.

6. ALL ALTERNATE CONCRETE MIX DESIGN AND TEST STRENGTHS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.

7. MAXIMUM VERTICAL DROP OF CONCRETE SHALL BE NO MORE THAN 2'-0" FROM END OF PLACEMENT DEVICE TO PLACEMENT SURFACE.

8. CONCRETE COVER AT REINFORCING SHALL BE AS FOLLOWS:

| | | |
|---|---|---|
| a. | CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: | 3" CLEAR |
| b. | EXPOSED TO EARTH OR WEATHER BUT CAST AGAINST FORMS | 2" CLEAR |
| c. | SLABS (EXCEPT FOR MATS) | REBAR AT CENTER OF SLAB |
| d. | BARS PARALLEL TO COLD JOINTS | 2" CLEAR |
| e. | NOT EXPOSED TO WEATHER OR EARTH SLABS, WALLS, JOISTS | ¾" CLEAR |
| f. | NOT EXPOSED TO WEATHER OR EARTH BEAMS AND COLUMN | 1½" CLEAR |

9. ALL REINFORCING STEEL, DOWELS, ANCHOR BOLTS, PIPE SLEEVES AND OTHER INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING OF CONCRETE. "WET SETTING" WILL NOT BE ALLOWED.

10. THE SURFACE OF ALL CONSTRUCTION JOINTS SHALL BE CLEANED AND ROUGHENED BY REMOVING THE ENTIRE SURFACE AND EXPOSING CLEAN AGGREGATE SOLIDLY EMBEDDED IN MORTAR MIX.

11. EPOXY SET ANCHORS SHALL BE INSTALLED IN CONCRETE THAT HAS A MINIMUM AGE OF 21 DAYS PER ACI D5.5.2.

12. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. THE ACCEPTABILITY OF CERTIFICATION OTHER THAN ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION SHALL BE THE RESPONSIBILITY OF THE LICENSED DESIGN PROFESSIONAL.

## REINFORCING BAR

1. REINFORCING STEEL SHALL BE DEFORMED BARS PER ASTM A615 WITH BAR MARKS LEGIBLY ROLLED INTO THE SURFACE INDICATION SIZE, TYPE OF STEEL, AND YIELD STRENGTH DESIGNATION:

| | | |
|---|---|---|
| a. | #3 BARS AND SMALLER | GRADE 40 OR GRADE 60 |
| b. | #4 BARS AND LARGER | GRADE 60 |
| c. | ALL BARS TO BE WELDED | GRADE A706 |

2. REINFORCING SHALL HAVE A MINIMUM LAP IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS SHOWN ON THESE DRAWINGS. STAGGER SPLICES WHENEVER POSSIBLE. VERTICAL WALL REINFORCING BARS SHALL EITHER EXTEND INTO FOOTINGS OR LAP SPLICED WITH FOOTING DOWELS OF THE SAME SIZE BARS.

3. BENDING OF REINFORCING SHALL BE IN CONFORMANCE WITH DETAILS AND SPECIFICATIONS SHOWN ON THESE DRAWINGS. FIELD BENDING OF BARS THAT ARE IN PLACE IS NOT PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

4. ALL BARS SHALL BE FREE OF LOOSE AND FLAKY RUST AND SCALE, GREASE, OR OTHER MATERIALS WHICH MIGHT AFFECT OR IMPAIR BOND.

5. WELDED WIRE MESH (WWF) SHALL CONFORM TO ASTM A-185, EXCEPT AT SLABS ON GRADE WHICH MAY BE GR40. USE 6x6 W10/10 AND LAP 12" MIN UON

## STRUCTURAL STEEL

1. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:

| | |
|---|---|
| OTHER SHAPES AND PLATES | ASTM A36 |
| ELECTRODES | ASTM E70XX |
| BASE PLATES | ASTM A36 |
| ANCHOR BOLTS | ASTM F1554, Fy=36 ksi |
| THREADED ROD | ASTM F1554, Fy=36 ksi |

2. IF MATERIAL DOES NOT CONFORM WITH THE ASTM STANDARDS LISTED IN THE STRUCTURAL DRAWINGS, MATERIAL TEST REPORTS OR REPORTS OF TESTS MADE BY THE FABRICATOR OR A TESTING LABORATORY SHALL CONSTITUTE SUFFICIENT EVIDENCE OF CONFORMITY WITH THE DESIGNATED ASTM STANDARDS LISTED IN AISC 360 SECTION A3.

3. ALL STEEL EXPOSED TO "WEATHER" OR PRESSURE TREATED LUMBER SHALL BE PROTECTED FROM CORROSION FOR THE FULL DURATION OF THE INSTALLATION WITH AN APPROVED METHOD OF PROTECTION INCLUDING, BUT NOT LIMITED TO:
   EPOXY PAINT
   HOT-DIPPED GALVANIZED
   IF STEEL IS PAINTED, PAINT MUST BE APPLIED AFTER WELDS ARE COMPLETE, AND PAINT SHALL BE MAINTAINED BASED ON APPROVED MAINTENANCE SCHEDULE. IF STEEL IS HOT-DIPPED GALVANIZED, "WELDING SLAG" MUST BE REMOVED AND ZINC-RICH PAINT MUST BE APPLIED TO WELD AND ADJACENT AREAS WHERE COATING HAS BEEN DAMAGED. ZINC-RICH PAINT MUST BE APPLIED TO A THICKNESS EQUIVALENT TO HOT-DIPPED GALVANIZED COATING

4. ALL STRUCTURAL STEEL SHALL CONFORM TO AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. BOLT HOLES SHALL BE 1/16" OVERSIZED, EXCEPT AT BASE PLATES, WHEN APPROVED, WHERE THEY CAN BE 5/16" OVERSIZED, WITH WELDED WASHERS.

5. ALL SHOP AND FIELD WELDING SHALL BE INSPECTED BY AN APPROVED TESTING LABORATORY. SPECIAL INSPECTION REQUIREMENTS OF CHAPTER 17, CBC, APPLY TO ALL WELDING.

6. ALL WELDING TO CONFORM TO THE REQUIREMENTS OF THE LATEST AWS D1.1 STRUCTURAL WELDING CODE AND SHALL BE PERFORMED BY CERTIFIED WELDERS.

7. ALL WELDS NOT SPECIFIED SHALL BE CONTINUOUS FILLET WELDS, USING NOT LESS THAN THE MINIMUM SIZES BASED ON THICKNESS OF THICKER PART JOINED PER AISC/AWS, AND IN NO CASE LESS THAN 1/4" UNLESS NOTED OTHERWISE.

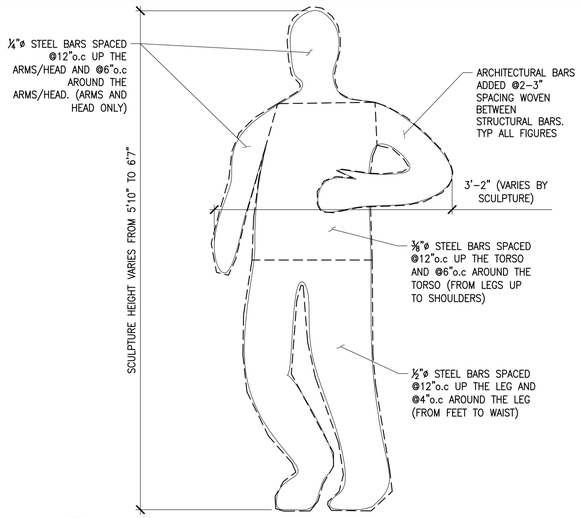
8. WHERE WELDS ARE DESIGNATED AS DEMAND CRITICAL, THEY SHALL BE MADE WITH A FILLER METAL CAPABLE OF PROVIDING A MINIMUM CHARPY V-NOTCH (CVN) TOUGHNESS OF 20 FT-LB AT -20°F AS DETERMINED BY THE APPROPRIATE AWS CLASSIFICATION TEST METHOD OR MANUFACTURER CERTIFICATION, AND 40 FT-LB AT 70°F AS DETERMINED BY APPENDIX X OR OTHER APPROVED METHOD, WHEN THE STEEL FRAME IS NORMALLY ENCLOSED AND MAINTAINED AT A TEMPERATURE OF 50°F OR HIGHER.

9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES ESPECIALLY WITH RELATION TO TEMPERATURE DIFFERENTIALS, ERECTION TOLERANCES, AND WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO REINFORCED CONCRETE WALLS.

10. THE STRUCTURAL STEEL CONNECTIONS CONSIST OF THE FOLLOWING:
    a. ALL MAJOR STRUCTURAL STEEL CONNECTIONS ARE DETAILED ON THE DRAWINGS. THE DETAILS INDICATE THE REQUIRED MINIMUM PLATE THICKNESSES, ANGLES, WELDS, BOLTS AND GENERAL CONNECTION CONFIGURATION. THE FINAL DIMENSIONAL CONFIGURATION INCLUDING ADJUSTMENTS FOR CAMBER SHALL BE DETERMINED BY THE FABRICATOR ON SHOP DRAWINGS.
    b. ANY PROPOSED REVISIONS OR MODIFICATIONS TO THE CONNECTIONS AS SHOWN ON THE DRAWINGS SHALL BE FULLY ENGINEERED BY THE FABRICATOR. SHOP DRAWINGS AND CALCULATIONS PREPARED AND STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA SHALL BE SUBMITTED FOR REVIEW. THE CAPACITY OF CONNECTIONS SHALL NOT BE REDUCED FROM THAT PROVIDED BY THE DETAIL AS SHOWN WHERE NOT SHOWN OR INFERRED FROM DRAWINGS, THE CONNECTION SHALL BE CAPABLE OF NOT LESS THAN 120% OF THE MEMBER CAPACITY IN TENSION. ANY PROPOSED REVISIONS SHALL BE AT NO ADDITIONAL COST TO THE OWNER.

10 GENERAL NOTES

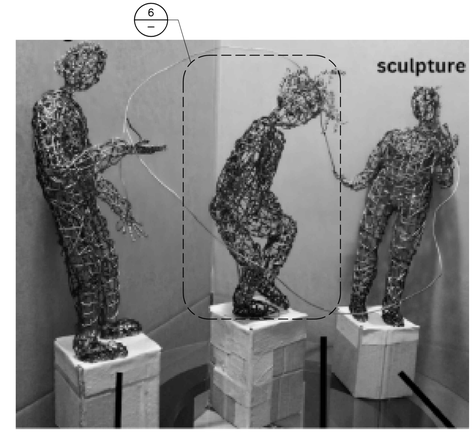
SCALE: NTS




NOTES:
1. ALL FIGURES CONSISTENT WITH ABOVE STEEL BAR SIZE DESIGNATIONS UNLESS OTHERWISE STATED

6 TYPICAL BAR SPACING DETAIL

SCALE: 1"=1'-0"

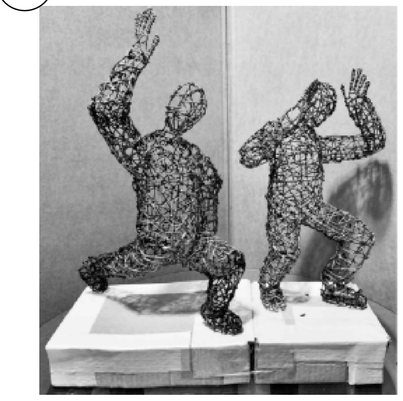



5 GIRLS DOUBLE DUTCH JUMP ROPE

SCALE: NTS

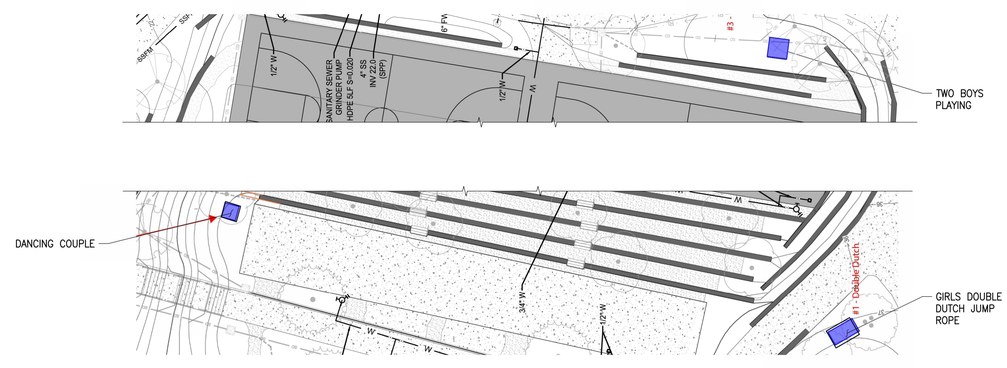

3 DANCING COUPLE

SCALE: NTS



2 TWO BOYS PLAYING

SCALE: NTS




1 ARTWORK LOCATION PLAN

SCALE: NTS

# INDIA BASIN SHORELINE PARK

CITY AND COUNTY OF SAN FRANCISCO
RECREATION AND PARKS DEPARTMENT
49 SOUTH VAN NESS AVENUE, SUITE 1220
SAN FRANCISCO, CA 94102
PH. 415-831-2700

THE TRUST FOR PUBLIC LAND
101 MONTGOMERY STREET
SUITE 900
SAN FRANCISCO, CA 94104
PH. 415-495-4014

ARTWORK ENGINEER
RBHU
PH. 925-212-4350

PRIME CONSULTANT / LANDSCAPE ARCHITECT
GGN
PH. 206-903-6802

CIVIL ENGINEER
SHERWOOD DESIGN ENGINEERS
PH. 415-348-9650

ARCHITECT
JENSEN ARCHITECTS
PH. 415-348-9650

ECOLOGICAL RESTORATION
RANA CREEK
PH. 831-659-3820

STRUCTURAL ENGINEER
JON BRODY STRUCTURAL ENGINEERS
PH. 415-296-9494

COASTAL ENGINEER
MOFFATT AND NICHOL
PH. 925-944-5411

LIGHTING
NITEO CALIFORNIA
PH. 415-666-2232

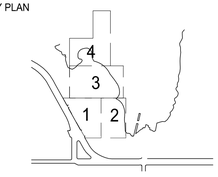
MEP & IT
INTERFACE ENGINEERING
PH. 415-489-7240

GEOTECHNICAL ENGINEER
AGS, INC
PH. 415-957-9240

SECURITY CONSULTANT
ZBETA CONSULTING
PH. 415-259-0422

FOR OFFICIAL USE

KEY PLAN




REGISTERED PROFESSIONAL ENGINEER
ALLINDA A. MAKLAKE
No. 82038
Exp. 3-31-28
CIVIL
STATE OF CALIFORNIA

REVISIONS:

| NO. | DATE | DESCRIPTION |
|---|---|---|
| | | |
| | | |
| | | |
| | | |

ISSUANCE

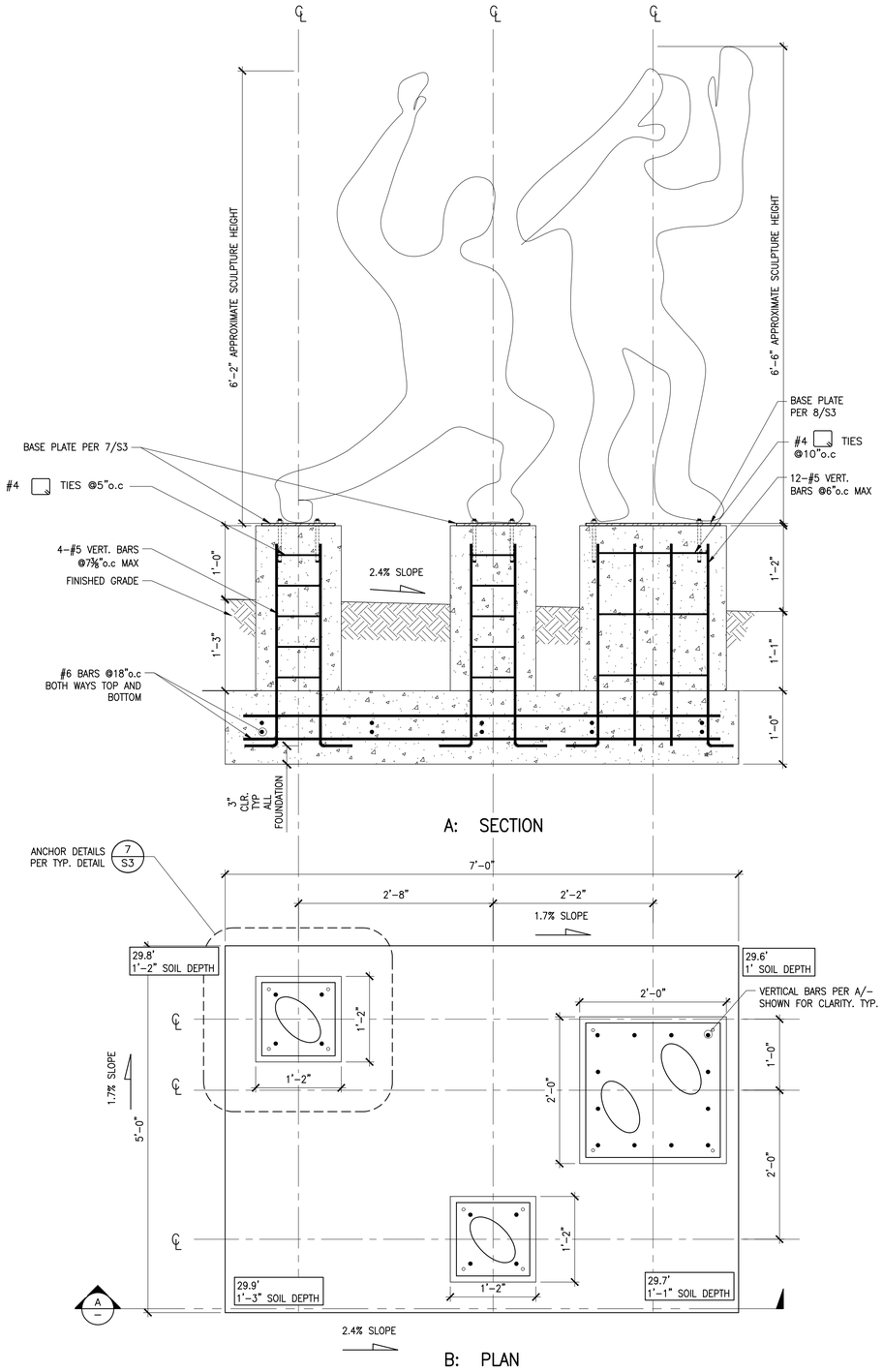
ISSUED FOR CONSTRUCTION

| DRAWN BY | CHECKED BY |
|---|---|
| CRC | AL |
| DATE | GGN PROJECT # |
| 07/11/2025 | 2366 |

General Notes and Sculpture Details

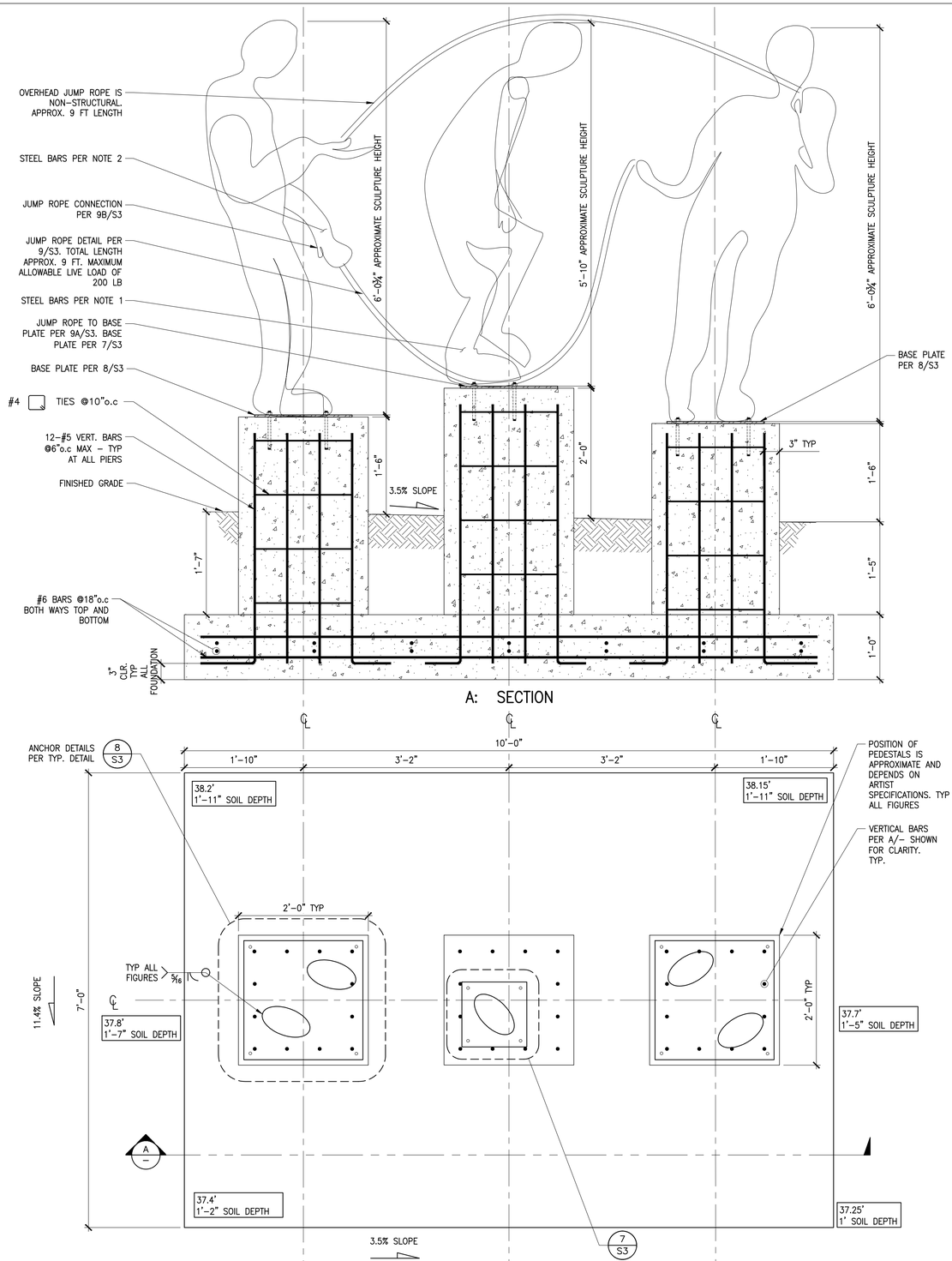
SA100.2

7/11/2025 ANSI A (8.50 x 11.00 Inches)




7 TWO BOYS PLAYING – FOUNDATION PLAN

SCALE: 3/4"=1'-0"




NOTES:
1. LIFTED LEG IN CENTER DOUBLE DUTCH FIGURE 1/4"ø BAR UP TO THIGH. SPACED @12"o.c UP THE LEG AND @6"o.c AROUND THE LEG
2. ARMS SUPPORTING JUMP ROPE IN DOUBLE DUTCH FIGURES 3/8"ø BAR UP TO TORSO. SPACED @10"o.c UP THE ARM AND @3"o.c AROUND THE ARM.

1 GIRLS DOUBLE DUTCH JUMP ROPE – FOUNDATION PLAN

SCALE: 3/4"=1'-0"

# INDIA BASIN SHORELINE PARK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RECREATION AND PARKS DEPARTMENT
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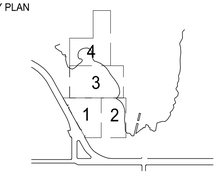
FOR OFFICIAL USE

KEY PLAN





REVISIONS:

| NO. | DATE | DESCRIPTION |
|---|---|---|
| | | |
| | | |
| | | |
| | | |

ISSUANCE

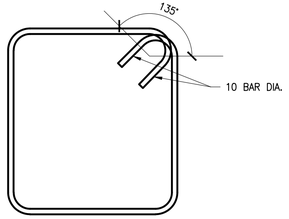
ISSUED FOR CONSTRUCTION

| DRAWN BY | CHECKED BY |
|---|---|
| CRC | AL |
| DATE | GGN PROJECT # |
| 07/11/2025 | 2366 |

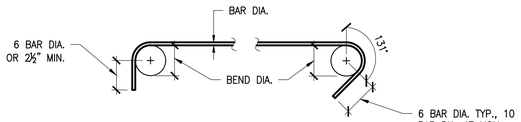
Foundation Plans and Details

SA101.2

7/11/2025 ANSI A (8.50 x 11.00 Inches)




TYPICAL STIRRUP (CLOSED–TIE)




90° HOOK  135° HOOK

NOTE:

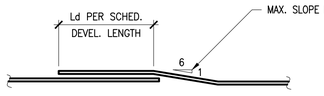
1. MINIMUM BEND DIAMETER FOR STIRRUPS AND TIES IS 4d FOR #5 BARS & SMALLER AND 6d FOR #6 THRU #8 BARS.
2. STIRRUPS CALLED OUT ON THE DRAWINGS MAY NEED TO BE INSTALLED IN PIECES TO ACCOMMODATE FIELD AND EXISTING CONDITIONS. WHERE THIS NEEDS TO OCCUR EACH PIECE NEEDS TO HAVE HOOKED ENDS AS SHOWN IN THIS DETAIL.
3. ON DRAWINGS, WHERE A STIRRUP CORNER SHOWS DOUBLE HOOKS, IT MEANS THAT HOOKS LAP AT CORNER. THIS THEREFORE IDENTIFIES THE PIECES OF THE STIRRUP

## 12 CONCRETE REBAR DETAILS STIRRUP & TIE BENDS

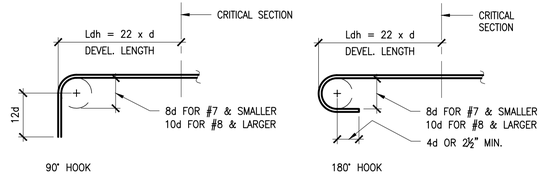
SCALE: N.T.S.

| SIZE | LOCATION | CONCRETE COMPRESSIVE STRENGTH (PSI) | | | | |
|---|---|---|---|---|---|---|
| | | 2,500 | 3,000 | 4,000 | 5,000 | 6,000 |
| #5 | HORIZ. TOP BAR | 51 | 47 | 41 | 36 | 33 |
| | ALL OTHER BARS | 39 | 36 | 31 | 28 | 26 |
| #6 | HORIZ. TOP BAR | 61 | 56 | 49 | 44 | 40 |
| | ALL OTHER BARS | 47 | 43 | 37 | 34 | 31 |

LAP SPLICE (Ld) SCHEDULE (INCHES)
CLASS B




LAP SPLICE



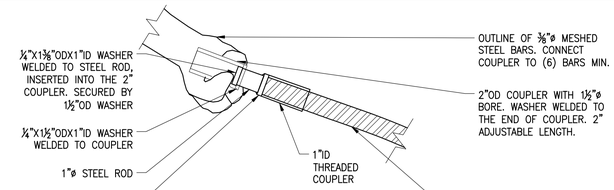

STANDARD HOOK

NOTES:

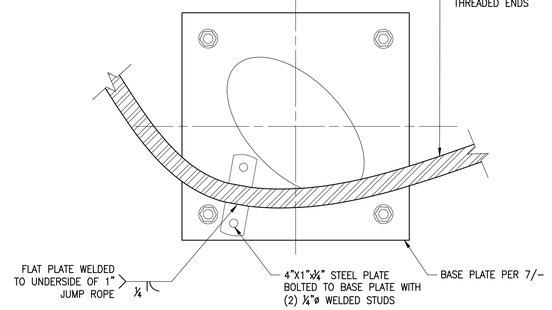
1. d = REBAR DIAMETER.
2. LAP SPLICE LENGTHS ARE BASED ON 60 KSI REBAR YIELD STRENGTH AND NORMAL CONCRETE WEIGHT.
3. TOP BAR IS A HORIZONTAL BAR (OTHER THAN IN WALLS) PLACED WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW BARS.
4. LAP SPLICE LENGTHS ARE BASED ON MINIMUM CLEAR COVER GREATER THAN ONE BAR DIAMETER AND MINIMUM CLEAR SPACING GREATER THAN TWO BAR DIAMETERS.
5. IF EITHER REQUIREMENT IN NOTE 4 IS NOT SATISFIED, INCREASE LAP SPLICE LENGTH BY 50%.

## 10 CONCRETE REBAR DETAILS REBAR DEVELOPMENT LENGTHS

SCALE: N.T.S.



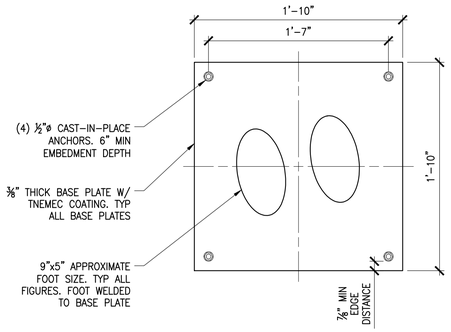

B: JUMP ROPE TO HAND




A: JUMP ROPE TO BASE PLATE

## 9 JUMP ROPE CONNECTION DETAILS

SCALE: 3"=1'–0"

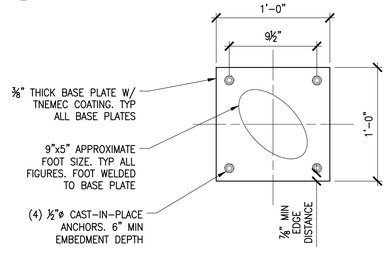



NOTES:

1. FOOT POSITIONING VARIES PER FIGURE. ANCHOR DETAILS INCLUDING EDGE DISTANCES AND SPACING REMAIN CONSISTENT.
2. 4 ANCHORS TYP. PER BASE PLATE

## 8 TYPICAL TWO FEET BASE PLATE DETAIL

SCALE: 1–1/2"=1'–0"

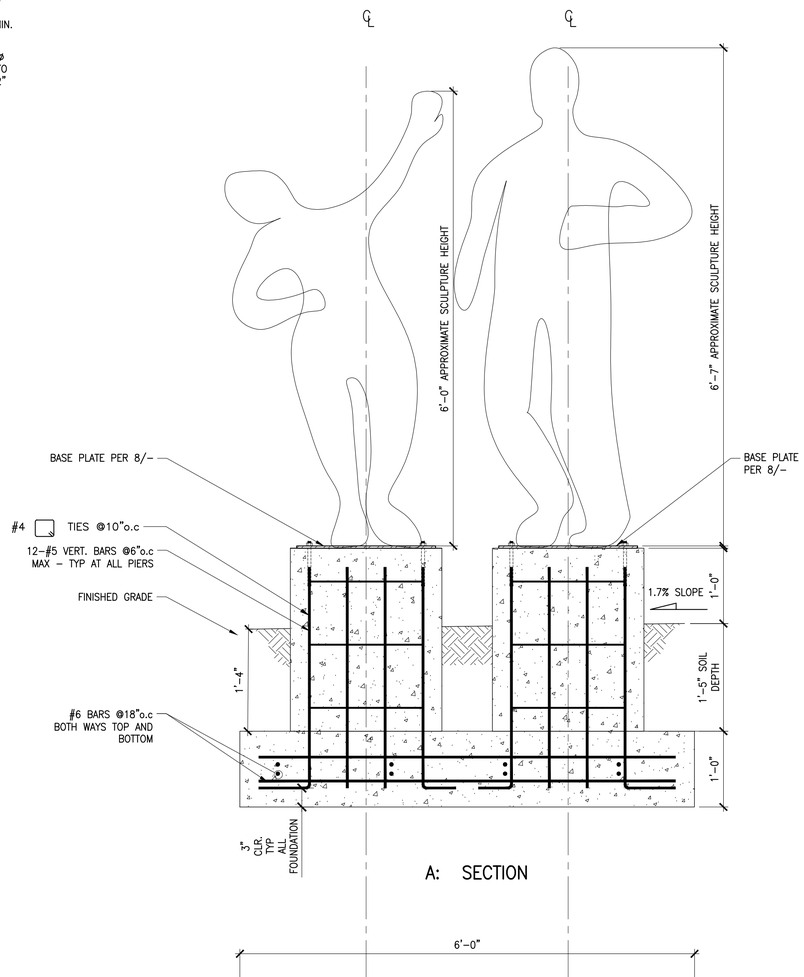



NOTES:

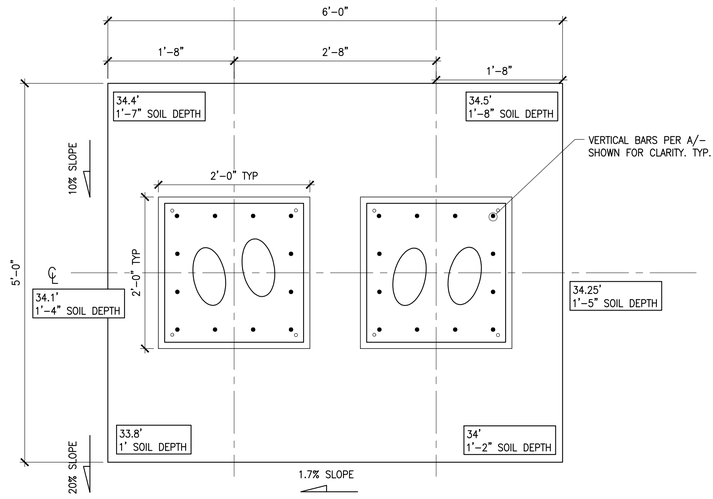
1. FOOT POSITIONING VARIES PER FIGURE. ANCHOR DETAILS INCLUDING EDGE DISTANCES AND SPACING REMAIN CONSISTENT.
2. 4 ANCHORS TYP. PER BASE PLATE

## 7 TYPICAL SINGLE FOOT BASE PLATE DETAIL

SCALE: 1–1/2"=1'–0"




A: SECTION




B: PLAN

## 1 DANCING COUPLE – FOUNDATION PLAN

SCALE: 3/4"=1'–0"

# INDIA BASIN SHORELINE PARK

CITY AND COUNTY OF SAN FRANCISCO
RECREATION AND PARKS DEPARTMENT
49 SOUTH VAN NESS AVENUE, SUITE 1220
SAN FRANCISCO, CA 94102
PH. 415-831-2700

THE TRUST FOR PUBLIC LAND
101 MONTGOMERY STREET
SUITE 900
SAN FRANCISCO, CA 94104
PH. 415-495-4014

ARTWORK ENGINEER
RBHU
PH. 925-212-4350

PRIME CONSULTANT / LANDSCAPE ARCHITECT
GGN
PH. 206-903-6802

CIVIL ENGINEER
SHERWOOD DESIGN ENGINEERS
PH. 415-348-9650

ARCHITECT
JENSEN ARCHITECTS
PH. 415-348-9650

ECOLOGICAL RESTORATION
RANA CREEK
PH. 831-659-3820

STRUCTURAL ENGINEER
JON BRODY STRUCTURAL ENGINEERS
PH. 415-296-9494

COASTAL ENGINEER
MOFFATT AND NICHOL
PH. 925-944-5411

LIGHTING
NITEO CALIFORNIA
PH. 415-666-2232

MEP & IT
INTERFACE ENGINEERING
PH. 415-489-7240

GEOTECHNICAL ENGINEER
AGS, INC
PH. 415-957-9240

SECURITY CONSULTANT
ZBETA CONSULTING
PH. 415-259-0422

FOR OFFICIAL USE

KEY PLAN



REVISIONS:

| NO. | DATE | DESCRIPTION |
|---|---|---|
| | | |
| | | |
| | | |
| | | |

ISSUANCE

ISSUED FOR CONSTRUCTION

| DRAWN BY | CHECKED BY |
|---|---|
| CRC | AL |
| DATE | GGN PROJECT # |
| 07/11/2025 | 2366 |

Foundation Plans and Details

SA102.2

7/11/2025 ANSI A (8.50 x 11.00 Inches)